

محضر استلام موقع

مشروع: أعمال الجسر الترابي والاعمال الصناعية لمشروع القطار الكهربائي السريع (العين السخنة - العاصمة الإدارية - العلمين - مطروح) قطاع فوكة مطروح (مرحلة الحفر وتشكيل الجسور والتأسيس والأساس والخرسانة) لتنفيذ المسافة من الكم 524+500 الى الكم 524+880 بطول 0.38 كم

تنفيذ: شركة المصطفى للمقاولات "أحمد مصطفى عبدالمحسن"

إشراف: المنطقة الخامسة - منطقة غرب الدلتا

طبقاً للعقد رقم (2024/2023/336) بتاريخ: 7/9/2023

إنه في يوم الخميس الموافق 7/9/2023 اجتمع كل من:-

- | | |
|--|---|
| 1- السيد المهندس / محمد حسني فياض | مدير عام المشروعات - الهيئة العامة للطرق والكباري |
| 2- السيد المهندس /إبراهيم عبد الله الحناوي | مهندس العملية - الهيئة العامة للطرق والكباري |
| 3- السيد المهندس / مصطفى محمد ثابت | مدير مشروع - شركة المصطفى للمقاولات |

وذلك للمرور على مسار العملية المذكورة عاليه لاستلام الموقع :-

وقد تبين أن الموقع خالياً من العوائق الظاهرية ويسمح بالبدء في التنفيذ وبناء عليه يعتبر تاريخ 2023/9/7 هو تاريخ استلام الموقع وبدء الأعمال بالعملية.

واقفل المحضر على ذلك ووقع الحضور

التوقيعات

3- السيد ثابت

2- السيد

1- السيد

رئيس الإدارة المركزية

منطقة غرب الدلتا

الاسكندرية - مرسى مطروح

عميد . مهندس /

"هاني محمد محمود طه"



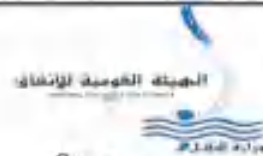
Owner Consultant



Contractor Consultant



Contractor



Owner

Compressive Strength For Stone Sample

Testing date	13-04-2023
Company Name	AL MUSTAFA
Location	524+500 To 524+800

Sample No.	Station	Weight (gm)	Volume (Cm3)	Density (gm/cm3)	Average Density (gm/cm3)	Load (KN)	Strength (Kg/Cm2)	Average Strength (Kg/Cm2)
1	524+500 To 524+560	999	421.88	2.37	2.31	249	450.69	463.4
2		915	421.88	2.17		132	238.92	
3		1007	421.88	2.39		387	700.47	
4	524+560 To 524+620	1042	421.88	2.47	2.22	395	466.495	318.5
5		948	421.88	2.25		299	353.119	
6		823.5	421.88	1.95		115	135.815	
7	524+620 To 524+680	714	343.00	2.08	2.17	117	243.36	251.8
8		726.8	343.00	2.12		166	345.28	
9		853	367.50	2.32		86	166.84	
10	524+680 To 524+740	841	421.88	1.99	1.97	75	135.75	152.0
11		866.6	421.88	2.05		86	155.66	
12		789	421.88	1.87		91	164.71	
13	524+740 To 524+800	850	421.88	2.48	2.53	80	144.8	131.5
14		802	343.00	2.55		80	124.8	
15		879	421.88	2.56		69	124.89	

Lab. Engineer

Name :

Sign :





Consultant Engineer

Name :

Sign :

Hassan

	Electric Express Train - NSR From El Aïn El Sokhra City To El Aïn El Matrouh - MATROUH Section - 7 From POHA TO MARSA MATROUH From Station 524+000 To Station 560+177		

Compressive Strength of stone sample

Testing Date :	25-2-2023		Station	524+500 To 524+750
Location :	K.P (524+500)		Material	Rock
Company Name :	المصطفى			

Upstream : -	AL Nuby Central Lab
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Sample No.	Station	wieght (gm)	density (gm/cm3)	Average Density	load (KN)	Strength (Kg/cm2)	Average Strength (Kg/cm2)
1	524+500 To 524+550	910	2.65	2.50	384	798.7	641.3
2		898	2.62		409	880.7	
3		766	2.23		132	278.6	
4	524+550 To 524+600	866.5	2.43	2.54	602	1252.2	1107.9
5		828	2.41		398	827.8	
6		921	2.69		598	1243.8	
7	524+600 To 524+650	980.9	2.86	2.69	510	1060.8	857.7
8		947.8	2.76		586	1218.9	
9		776	2.16		141	293.3	
10	524+650 To 524+700	943.4	2.75	2.73	699	1453.9	1248.4
11		938	2.73		513.6	1068.3	
12		931	2.71		588	1229.0	
13	524+700 To 524+750	850.4	2.48	2.58	634	1318.7	1218.2
14		889.4	2.59		645	1341.8	
15		912	2.66		478	996.2	

Lab. Engineer
 Name :
 Sign :



Consultant Engineer
 Name :
 Sign :





اختبار الضغط على عينات حجر

- الجهة : وزارة النقل - الهيئة العامة للطرق والكبارى والنقل البرى - المنطقة الخامسة - غرب الدلتا الاسكندرية - مطروح
- الشركة المنفذة: المصطفى للمقاولات
- الاشراف: المنطقة الخامسة - غرب الدلتا (اسكندرية - مطروح)
- المشروع: القطار الكهربائى السريع القطاع السابع فوكه - مطروح
- توريد العينات: 2023-2-6

عينة رقم (2)

(من المحطة 524+540 الى المحطة 524 + 580)

نتائج الاختبار :-

رقم	أبعاد العينة (سم)			مساحة المقطع (سم ²)	وزن العينة (جم)	حمل الكسر (kN)	إجهاد الكسر (كجم/سم ²)	ملاحظات
	طول	عرض	ارتفاع					
1	7.0	7.0	6.8	49.0	850	265	551.3	-----
2	7.0	6.8	7.0	47.6	772	105	224.9	
3	7.0	6.9	7.0	48.3	805	283	597.3	

ملحوظة هامة:-

- تم تحديد عدد العينات (3) بمعرفة العميل ، مع العلم أن هذا العدد غير مطابق للمواصفات القياسية لهذا الاختبار
- تم توريد العينات بمعرفة العميل وكذلك المعلومات الخاصة بها .

مدير المعمل

د. عبد اللطيف السيد أبوسن

المشرف على الاختبار

د. إسماعيل أحمد محمد محروس

تحريرا فى : 2023/2/9

رقم التقرير:-

٢٠٤/٩٦٤





اختبار الضغط على عينات حجر

- الجهة : وزارة النقل - الهيئة العامة للطرق والكبارى والنقل البرى - المنطقة الخامسة - غرب الدلتا الاسكندرية - مطروح
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- توريد العينات: 2023-2-6

عينة رقم (3)

(من المحطة 524+580 الى المحطة 524 + 620)

نتائج الاختبار :-

رقم	أبعاد العينة (سم)			مساحة المقطع (سم ²)	وزن العينة (جم)	حمل الكسر (kN)	إجهاد الكسر (كجم/سم ²)	ملاحظات
	طول	عرض	ارتفاع					
1	7.0	7.0	7.1	49.0	873	293	609.5	-----
2	7.0	7.0	7.0	49.0	870	209	434.8	
3	7.0	7.0	7.1	49.0	845	257	534.6	

ملحوظة هامة:-

- تم تحديد عدد العينات (3) بمعرفة العميل ، مع العلم أن هذا العدد غير مطابق للمواصفات القياسية لهذا الاختبار .
- تم توريد العينات بمعرفة العميل وكذلك المعلومات الخاصة بها .

مدير المعمل

د. عبد اللطيف السيد أبوسن

المشرف على الاختبار

د. إسماعيل أحمد محمد محروس

تحريرا فى : 2023/2/9

رقم التقرير :-

٩٧١ / ٢٢٢





اختبار الضغط على عينات حجر

- الجهة : وزارة النقل - الهيئة العامة للطرق والكبارى والنقل البرى - المنطقة الخامسة -
غرب الدلتا الاسكندرية - مطروح
الشركة المنفذة: المصطفى للمقاولات
الإشراف: المنطقة الخامسة - غرب الدلتا (اسكندرية - مطروح)
المشروع: القطار الكهربائى السريع القطاع السابع فوكه - مطروح
توريد العينات: 2023-2-6

عينة رقم (4)

(من المحطة 620 + 524 الى المحطة 660 + 524)

نتائج الاختبار :-

رقم	أبعاد العينة (سم)			مساحة المقطع (سم ²)	وزن العينة (جم)	حمل الكسر (kN)	إجهاد الكسر (كجم/سم ²)	ملاحظات
	طول	عرض	ارتفاع					
1	7.0	7.0	7.0	49.0	786	153	318.3	-----
2	7.0	7.0	6.9	49.0	816	296	615.8	
3	7.0	7.0	7.1	49.0	855	249	518.0	

- ملحوظة هامة:

- تم تحديد عدد العينات (3) بمعرفة العميل ، مع العلم أن هذا العدد غير مطابق للمواصفات القياسية لهذا الاختبار .
- تم توريد العينات بمعرفة العميل وكذلك المعلومات الخاصة بها .

مدير المعمل

د. عبد اللطيف السيد أبوسن

المشرف على الاختبار

د. إسماعيل أحمد محمد محروس

تحريرا فى : 2023/2/9

رقم التقرير:

٩٣٢/٩٣٢





اختبار الضغط على عينات حجر

- الجهة : وزارة النقل - الهيئة العامة للطرق والكبارى والنقل البرى - المنطقة الخامسة - غرب الدلتا الاسكندرية - مطروح
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- توريد العينات: 2023-2-6

عينة رقم (5)

(من المحطة 660 + 524 الى المحطة 700 + 524)

نتائج الاختبار:-

ملاحظات	إجهاد الكسر (كجم/سم ²)	حمل الكسر (kN)	وزن العينة (جم)	مساحة المقطع (سم ²)	أبعاد العينة (سم)			رقم
					ارتفاع	عرض	طول	
-----	507.6	244	877	49.0	6.9	7.0	7.0	1
	470.2	226	868	49.0	7.0	7.0	7.0	2
	654.3	319	825	49.7	6.9	7.0	7.1	3

ملحوظة هامة:

- تم تحديد عدد العينات (3) بمعرفة العميل ، مع العلم أن هذا العدد غير مطابق للمواصفات القياسية لهذا الاختبار
- تم توريد العينات بمعرفة العميل وكذلك المعلومات الخاصة بها .

مدير المعمل

د. عبد اللطيف السيد أبومن

المشرف على الاختبار

د. إسماعيل أحمد محمد محروس

تحريرا فى : 2023/2/9

رقم التقرير:

٩٦٣/٢٠٢٣





اختبار الضغط على عينات حجر

- الجهة : وزارة النقل - الهيئة العامة للطرق والكبارى والنقل البرى - المنطقة الخامسة -
غرب الدلتا الاسكندرية - مطروح
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- المشروع: القطر الكهربائى السريع القطاع السابع فوكه - مطروح
- توريد العينات: 2023-2-6

عينة رقم (1)

(من المحطة 524+500 الى المحطة 524 + 540)

نتائج الاختبار :-

رقم	أبعاد العينة (سم)			مساحة المقطع (سم ²)	وزن العينة (جم)	حمل الكسر (kN)	إجهاد الكسر (كجم/سم ²)	ملاحظات
	طول	عرض	ارتفاع					
1	7.0	6.8	7.0	47.60	793	196	419.7	-----
2	7.0	6.5	7.0	45.50	767	196	439.1	
3	6.9	6.9	7.0	47.61	765	135	289.0	

ملحوظة هامة:-

- تم تحديد عدد العينات (3) بمعرفة العميل ، مع العلم أن هذا العدد غير مطابق للمواصفات القياسية لهذا الاختبار
- تم توريد العينات بمعرفة العميل وكذلك المعلومات الخاصة بها .

مدير المعمل

د. عبد اللطيف السيد أبوسن



المشرف على الاختبار

د. إسماعيل أحمد محمد محروس

تحريرا فى : 2023/2/9

رقم التقرير:-

٩٥٩ / ٢٠٢٣

MATERIAL INSPECTION REQUEST	    				

Contractor Company	AL-MOSTAFA COMPANY		Designer Company	K.K																
Issued by Contractor	Name	Sign	Date	Time																
	MOSTAFA THABET	<i>Mostafa Thabet</i>	26-02-2023																	
Received by ER			MIR	<table border="1"> <tr> <td>1</td> <td>2</td> <td>3</td> <td>DO</td> <td>MM</td> <td>YY</td> <td>HH</td> <td>MM</td> </tr> <tr> <td>K.P</td> <td>E.M</td> <td>D.T</td> <td>26</td> <td>02</td> <td>2023</td> <td></td> <td></td> </tr> </table>	1	2	3	DO	MM	YY	HH	MM	K.P	E.M	D.T	26	02	2023		
1	2	3	DO	MM	YY	HH	MM													
K.P	E.M	D.T	26	02	2023															

CODE - 1	S1 to S21 Station Reference	D1 to S3 Depot Reference	Kp XXX Note For Kilometer point only Start Km is used
CODE - 2	Work Activity		
CODE - 3	Sub Element of Activity		

Description of Materials	REPLACEMENT FILL MATERIAL RESULTS					
Location to be Used:	From	To				
	524+900	524+920	FILL (-3.00 m)			
	524+880	524+920	FILL (-2.50 m)			
	524+880	524+920	FILL (-2.00 m)			
	524+820	524+920	FILL (-1.75 m)			
	524+820	524+920	FILL (-1.50 m)			
	524+820	524+920	FILL (-1.25 m)			
	524+800	524+920	FILL (-1.00 m)			
	524+800	524+920	FILL (-0.75 m)			
	524+800	524+920	FILL (-0.50 m)			
MAR Approval No				Date		
Supplier Name						
Test Requirement	Specification			Clause		
Reference Photos	Yes attached / No			Other		
Item	Description	Unit	Quantity	Arrival Date	Note	
1	Sieve analysis	M3	5000	26-02-2023		
2	Classification	M3	5000	26-02-2023		
3	Proctor & O.M.C	M3	5000	26-02-2023		
4	L.L & P.L & PI	M3	5000	26-02-2023		
5	C.B.R	M3	10000	26-02-2023		
Comments by:			Comments by:			
A sample has been taken from fill material by K.K office to (GOUMA BADR LAB) and the results founded meet the specifications and accepted.						
APPROVAL STATUS						
Organisation	Name	Sign	Date	A-AWC-R		
Contractor	<i>Mostafa Thabet</i>	<i>Mostafa Thabet</i>				
QA/QC *	<i>Abdullah SANY</i>	<i>Abdullah</i>				
GARB**						
Employers Representative						

SUBMISSION of TEST RESULTS

الهيئة العامة للإسكان



Contractor Company	AL-MOSTAFA COMPANY		Designer Company	K.K																
Issued by Contractor	Name	Sign	Date	Time																
	Mostafa Thabet	<i>Mostafa Thabet</i>	28-02-2023																	
Received by ER			STR	<table border="1"> <tr> <td>C1</td> <td>C2</td> <td>C3</td> <td>D0</td> <td>MM</td> <td>YY</td> <td>H</td> <td>MM</td> </tr> <tr> <td>K.P 524</td> <td>E.W</td> <td>Q.T</td> <td>28</td> <td>02</td> <td>2023</td> <td></td> <td></td> </tr> </table>	C1	C2	C3	D0	MM	YY	H	MM	K.P 524	E.W	Q.T	28	02	2023		
C1	C2	C3	D0	MM	YY	H	MM													
K.P 524	E.W	Q.T	28	02	2023															

CODE - 1	S1 to S21 Station Reference	D1 to S3 Depot Reference	Kp XXX Note For Kilometer point only Start Km is used
CODE - 2	Work Activity		
CODE - 3	Sub Element of Activity		

NB: Package 1 Only (Package 2 via Aconex)				
THE FOLLOWING TEST RESULTS ARE ATTACHED FOR REVIEW				
Description of Test Materials	Soil (A-1-b)			
Location of Test	K.P (524)			
Item	Specification	Test Requirement	Test Result Attachment	Remarks
1	ASTM D 75	Aggregate sampling	According to specification	
2	ASTM C 136	Sieve Analysis	According to specification	
3	ASTM D 1440	Passing sieve #200	13.2	
4	ASTM D 4318	Atterberg limit	N.P	
5	ASTM D 2974	Moisture content	6.3	
6	ASTM D 1557	Modified proctor	2.16	
7	ASTM D 1883	C.B.R	53.0	

Comments by:	Comments by:

APPROVAL STATUS				
Organisation	Name	Sign	Date	A-AWC-R
Contractor	<i>Mostafa Thabet</i>			A
Designer	<i>Hassan</i>	<i>[Signature]</i>		A
GARB *				
Employers Representative				

* Alignment / Bridges: Culvert Only

California Bearing Ratio TEST

Testing Date:	1/3/2023	Code:	FROM STA	Sample:	525-000
Location:	K.P (524+800)	MO (2)			
Company Name:	AL Mustafa				

Test Results

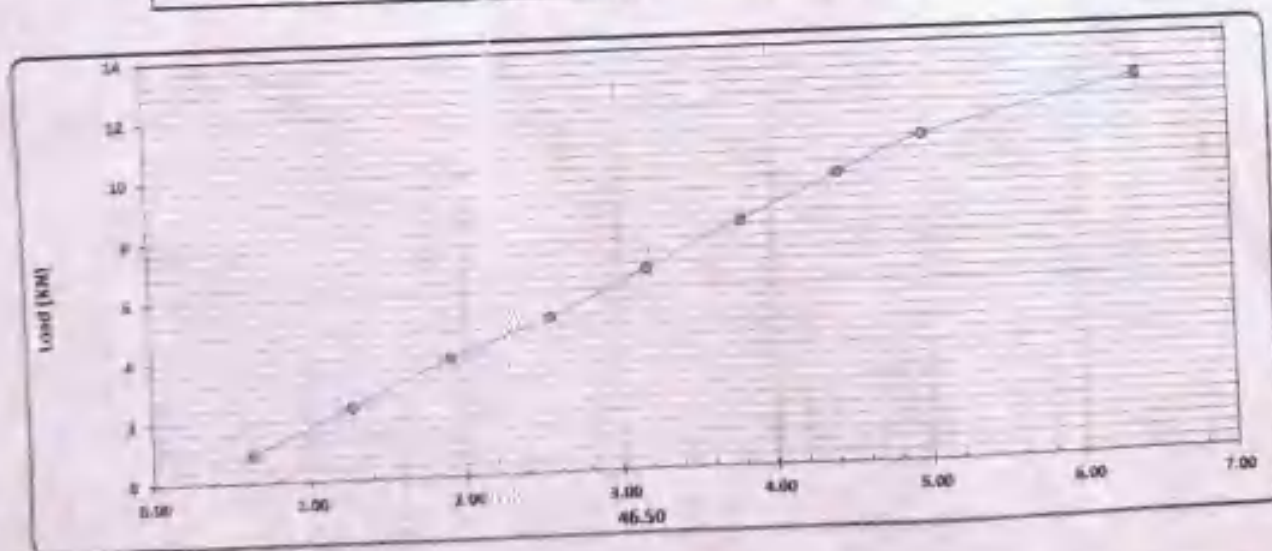
Compaction % for Mold	
Mold No.	1
Mold Vol. (cm ³)	2158
Mold WT. (gm)	6890
Mold WT. + Wet WT. (gm)	9811
Wet WT. (gm)	4921
Wet Density (g/cm ³)	2.280
Dry Density (g/cm ³)	2.154
Theoretical Density (g/cm ³)	2.180
Compaction %	99.7

Moisture Ratio After Compacted Mold	
Tare No.	18
Tare WT. (gm)	43.7
Tare WT. + Wet WT. (gm)	130
Tare WT. + Dry WT. (gm)	144.3
Wet	5.9
Dry WT. (gm)	100.4
Moisture Content %	5.9

Soil Boring	
Mold No.	1
Date:	1/3/2023
Initial Height (mm)	5.00
Final Height (mm)	5.15
Difference	0
Sample Height (mm)	120.00
Swelling Ratio %	0.1%

Loading Reading:

46.50	0.64	1.27	1.91	2.54	3.18	3.81	4.45	5.00	5.60
Load Reading (mm)	0.03	0.08	0.13	0.17	0.22	0.27	0.32	0.38	0.43
Load (KN)	0.8	2.4	3.9	5.1	6.6	8.1	9.6	10.8	12.8



Calculations :-

Penetration (mm)	Load (KN)	Standard Load (kN)	CBR (%)	Mold Compaction (%)	Compaction (%)	CBR (%)
2.50	0.17	13.4	0.25	100	99	37.5%
5.00	1.08	20.0	5.40			53.0%

Lab. Specialist

Name:

Sign:

Lab. Engineer

Name:

Sign:

Consultant Engineer

Name:

Sign:



California Bearing Ratio TEST

Testing Date :	1/3/2023	Code			
Location :	K.P (524+800)	MO (2)	18000 NIS	124-000	525+000
Company Name :	AL Mustafa				

Test Results

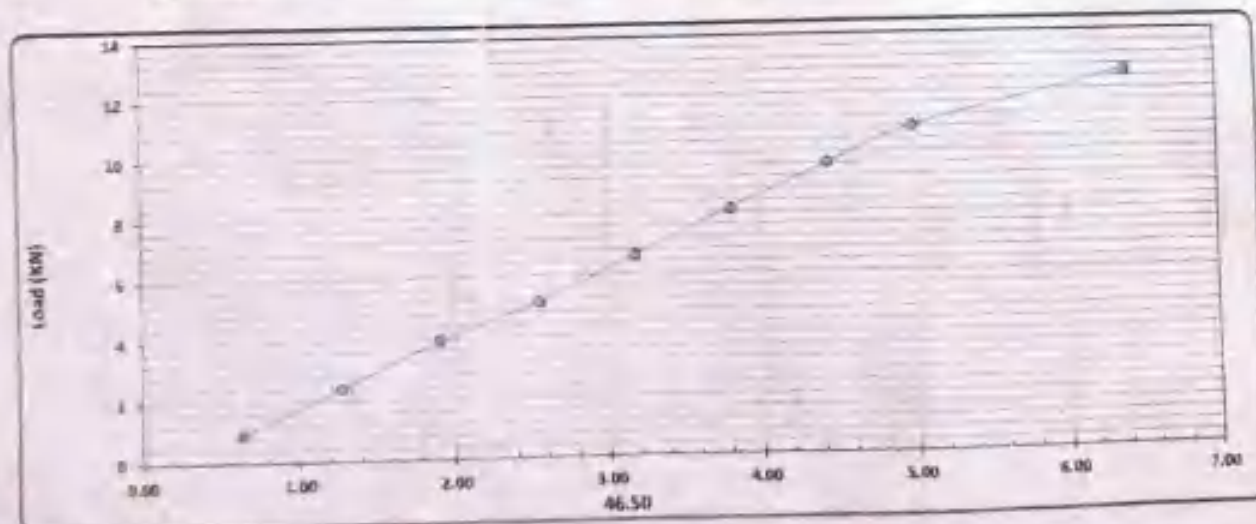
Compaction % for Mold	
Mold No.	1
Mold Vol. (cm ³)	1150
Mold WT. (gm)	4890
Mold WT. - Wet WT. (gm)	981
Wet WT. (gm)	4921
Wet Density (g/cm ³)	1.280
Dry Density (g/cm ³)	2.194
Proctor Density (g/cm ³)	2.190
Compaction %	99.7

Moisture Ratio After Compacted Mold	
Tare No.	10
Tare WT. (gm)	43.7
Tare WT. - Wet WT. (gm)	150
Tare WT. - Dry WT. (gm)	144.1
Moisture Ratio	5.9
Dry WT. (gm)	106.4
Moisture Content %	5.9

Swelling	
Mold No.	1
Date	1/3/2023
Initial Height (mm)	5.00
Final Height (mm)	5.15
Difference	0.15
Sample Height (mm)	120.00
Swelling Ratio %	0.1 %

Loading Reading :

46.50	0.64	1.27	1.91	2.54	3.18	3.81	4.45	5.09	5.73
Load Reading (mm)	0.03	0.06	0.13	0.17	0.22	0.27	0.32	0.36	0.42
Load (KN)	0.9	1.4	1.9	2.1	2.6	3.1	3.6	4.1	4.6



Calculations :-

Penetration	Load	Standard Load	CBR	Mold - Compaction	Compaction	CBR
(mm)	(KN)	(lb)	(%)	(%)	(%)	(%)
2.50	0.9	15.4	18.8%	100	98	37.5%
5.00	1.9	20.0	23.0%			53.0%

Lab. Specifier

Name :

Sign :

Lab. Engineer

Name :



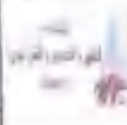
Sign :



Consultant Engineer

Name :

Sign :

 K.K. KONSULTING شركة الاستشارات الهندسية ك. ك. كونسولت Engineering Lab	 AL Nuby Central Lab	Electric Express Train - HSR From El Ain El Sokhna City To El Alamein - MATROUH Section - 7 From FOKA To MARSA MATROUH From Station 504+000 To Station 505+177	 وزارة التخطيط والتنمية الاقتصادية Ministry of Planning and Economic Development
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PARTICLE SIZE DISTRIBUTION OF SOIL

TESTING DATE:	26-2-2023	Code	zone	524+500	525+000
LOCATION	K.P (524+800)	MO (2)			
NAME COMPANY	AL Mustafa				

1-visual inspection test

2-Gradient test

A-gradation of bulk materials				SAMPLE WEIGHT (g)		25245.00		gm	table classify	
sieve size	2	1.5	1	4/3	2/1	8/3	# 4	PASS	soil classify	
Mass retained (g)	0.0	2215.0	2143.0	1987.0	2104.0	2201.0	2141.0		A-1-b	
Cumulative Retained (g)	0.0	2215.0	4357.0	6344.0	8448.0	10649.0	12790.0		PRO	2.15
Cumulative Retained %	0.0	8.8	17.3	25.1	33.5	42.2	50.7		WC	6.30
Cumulative Passing %	100.0	91.2	82.7	74.9	66.5	57.8	49.3		CBR	53%

B-soft material gradation				WT.OF sample		500.00		gm
sieve size	10	40	200					
Cumulative Retained (g)	25.60	160.00	346.70					
Cumulative Retained %	5.12	32.00	73.34					
Cumulative Passing %	94.88	68.00	26.66					

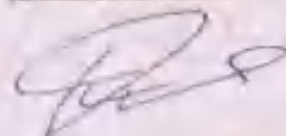
C-General gradient										
sieve size(in)	2	1.5	1	3/4	1/2	3/8	# 4	# 10	# 40	# 200
sieve size(mm)	50.0	37.5	25.0	19.0	12.5	9.5	4.75	2.00	0.425	0.075
Cumulative Passing %	100.0	91.2	82.7	74.9	66.5	57.8	49.3	38.7	20.8	13.2

ATTENDING LIMITS	LIQUID LIMIT (LL)	PLASTIC LIMIT (PL)	FLASTIC INDEX (FI)
	N.P	N.P	N.P

Contractor



Consultant





Electric Express Train - HSR
From El Ain El Sekhna City To El Alamein - MATROUH
Section - 7 From FOKA TO MARESA MATROUH
 From Station 524+800 To Station 525+000



PROCTOR TEST

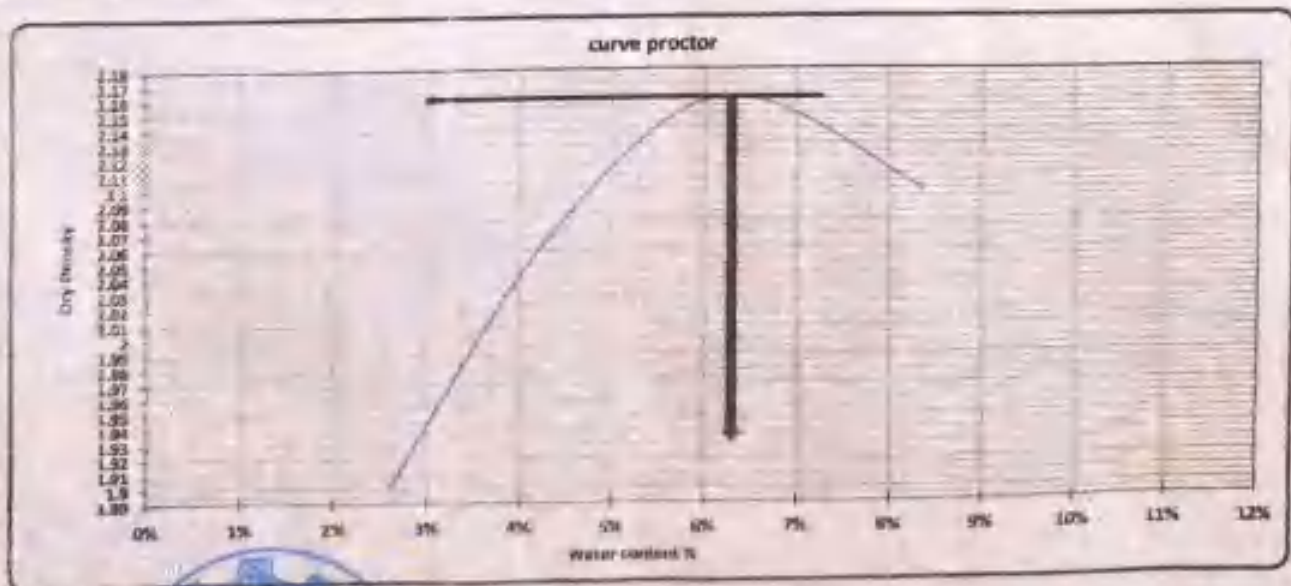
TESTING DATE:	26-2-2023	Code			
LOCATION	K.P (524+800)		zone	524+500	525+000
NAME COMPANY	AL Mustafa	MO (7)			

Weight of empty mold :	6037.0
Mold Volume:	2193.0

MAX Dry Density	2.16
Water content %	6.3

trial no :	1	2	3	4	
Wt. Of Mold+ wet soil	10145.0	10575.0	10865.0	10821	
WT. WET SOIL	4108.0	4538.0	4828.0	4784.0	
Wt. Density	1.953	2.158	2.296	2.275	

Tare No.	20	18	73	7	10	8	11	20		
Tare wt.	60.0	77.7	42.5	42.6	43.3	46.7	46.3	60.3		
Wt. Of wet soil & tare	150.0	150.0	150.0	150.0	150.0	150.0	150.0	150.0		
Wt. Of dry soil & tare	146.7	143.1	145.5	145.6	143.7	144.0	141.9	143.2		
Wt. Of water	3.3	6.9	4.5	4.4	6.3	6.0	8.1	6.8		
Wt. Of dry soil	123.0	127.8	103.0	103.0	100.4	97.3	95.6	82.9		
Water content %	2.7%	5.4%	4.4%	4.3%	6.3%	6.2%	8.5%	8.2%		
AV. Water content %	2.6%		4.3%		6.2%		8.3%			
Dry Density	1.903		2.069		2.161		2.100			



Consultant
 Hassan
 Signature: [Handwritten signature]

MATERIAL INSPECTION REQUEST

الهيئة العامة للإستشارة

General Consultancy Authority



Contractor Company	AL-MOSTAFA COMPANY			Designer Company	K.K		
Issued by Contractor	Name	Sign	Date	Time			
	MOSTAFA THABET	<i>Mostafa Thabet</i>	26-03-2023				
Received by ER			MIR	C1	C2	C3	DD
				K.P	C.W	O.T	26
				324			03
							2023

CODE-1	S1 to S21 Station Reference	D1 to S3 Depot Reference	Kp XXX Note For Kilometer point only Start Km is used
CODE-2	Work Activity		
CODE-3	Sub Element of Activity		

Description of Materials		REPLACEMENT FILL MATERIAL RESULTS				
Location to be Used	From	TO				
	524+900	524+920	FILL (-0.25 m)			
	524+880	524+920	FERMA			
	524+920	525+000	FILL (-3.00 m)			
	524+920	525+000	FILL (-2.50 m)			
	524+920	525+000	FILL (-2.00 m)			
MAR Approval No					Date	
Supplier Name						
Test Requirement		Specification			Clause	
Reference Photos		Yes attached / No			Other	
Item	Description	Unit	Quantity	Arrival Date	Note	
1	Sieve analysis	M3	5000	26-03-2023		
2	Classification	M3	5000	26-03-2023		
3	Proctor & O.M.C	M3	5000	26-03-2023		
4	L.L & P.I & PI	M3	5000	26-03-2023		
5	C.B.R	M3	10000	26-03-2023		
Comments by:			Comments by:			
A sample has been taken from fill material by K.K office to (GOUMA BADR LAB) and the results founded meet the specifications and accepted.						
APPROVAL STATUS						
Organisation	Name	Sign	Date	A-AWC-R		
Contractor	<i>Mostafa Thabet</i>	<i>Mostafa Thabet</i>				
QA/QC *	<i>Abdallah SAMY</i>	<i>Abdallah</i>				
GARB**						
Employers Representative						

SUBMISSION of TEST RESULTS

الهيئة العامة للغذاء والدواء
SAGRA



Contractor Company	AL-MOSTAFA COMPANY		Designer Company	K.K																
Issued by Contractor	Name	Sign	Date	Time																
	Mostafa Thabet	<i>Mostafa Thabet</i>	28-03-2023																	
Received by ER			STR	<table border="1"> <tr> <td>C1</td> <td>C2</td> <td>C3</td> <td>OD</td> <td>MM</td> <td>YY</td> <td>FF</td> <td>MM</td> </tr> <tr> <td>K.P. 524</td> <td>E.W</td> <td>O.T</td> <td>28</td> <td>03</td> <td>2023</td> <td></td> <td></td> </tr> </table>	C1	C2	C3	OD	MM	YY	FF	MM	K.P. 524	E.W	O.T	28	03	2023		
C1	C2	C3	OD	MM	YY	FF	MM													
K.P. 524	E.W	O.T	28	03	2023															

CODE - 1	S1 to S21 Station Reference	D1 to S3 Depot Reference	Kp XXX Note For Kilometer point only Start Km is used
CODE - 2	Work Activity		
CODE - 3	Sub Element of Activity		

NB: Package 1 Only (Package 2 via Aconex)				
THE FOLLOWING TEST RESULTS ARE ATTACHED FOR REVIEW				
Description of Test Materials		Soil (A-1-a)		
Location of Test		K.P (524)		
Item	Specification	Test Requirement	Test Result Attachment	Remarks
1	ASTM D 75	Aggregate sampling	According to specification	
2	ASTM C 136	Sieve Analysis	According to specification	
3	ASTM D 1440	Passing sieve #200	12.8	
4	ASTM D 4318	Atterberg limit	N.P	
5	ASTM D 2974	Moisture content	6.4	
6	ASTM D 1557	Modified proctor	2.16	
7	ASTM D 1883	C.B.R	56.0	

Comments by:	Comments by:

APPROVAL STATUS				
Organisation	Name	Sign	Date	A-AWC-R
Contractor	<i>Mostafa Thabet</i>			A
Designer	<i>Hassan</i>	<i>[Signature]</i>		A
GARB *				
Employers Representative				

* Alignment / Bridges; Culvert Only



Electric Express Train - HSR

California Bearing Ratio TEST

TESTING DATE	28/3/2023	ZONE	S24+500	S25+000
Location	K.P524+800			
NAME COMPANY	Al Moustafa			

Test Results

operate by GOMAA RADER LAB

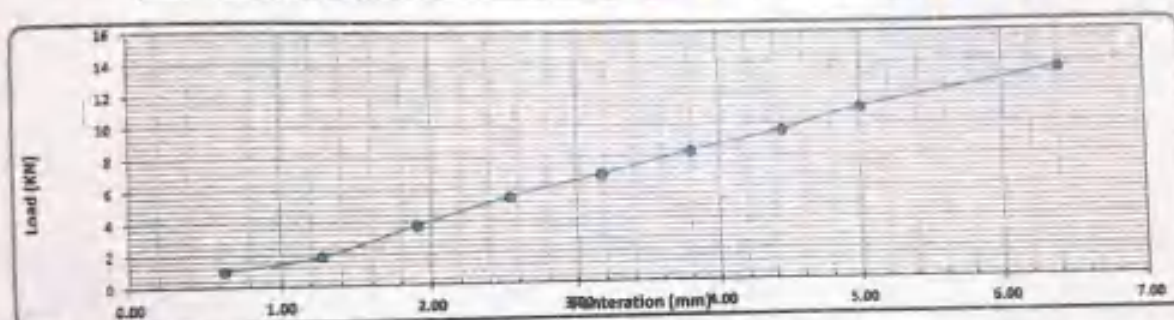
Compaction % for Mold	
Mold No.	1
Mold Vol. (cm ³)	2172.4
Mold WT. (gm)	10404
Mold WT. - Wet WT. (gm)	21340
Wet WT. (gm)	4902
Wet Density (g/cm ³)	2.251
Dry Density (g/cm ³)	1.111
Proctor Density (g/cm ³)	2.168
Compaction %	98

Moisture Ratio After Compacted Mold	
Test No.	4
Tare WT. (gm)	24.61
Test WT. + Wet WT. (gm)	196.45
Tare WT. + Dry WT. (gm)	188.86
WT. Of water	7.6
Dry WT. (gm)	114.3
Moisture Content %	6.1

Swelling	
Mold No.	2
Date	
Initial Height (mm)	
Final Height (mm)	
Difference	0
Sample Height (mm)	
Swelling Ratio %	

Loading Reading :

penetration	0.64	1.27	1.91	2.54	3.18	3.80	4.45	5.00	6.40
Load Reading (kg)	97.00	187.00	279.00	373.00	497.00	645.00	978.00	1176.00	
Load (KN)	1.0	1.8	3.7	4.7	6.8	8.3	9.6	11.0	13.5



Calculations :-

Penetration (mm)	Load (Kg)	Standard Load (lb)	CBR (%)	Mold - Compaction (%)	Compaction (%)	CBR % 100 نسبة
2.50	3.47	13.4	41.0%	98	100	41.7%
5.00	11.02	28.8	55.0%			56.0%

Lab. Specialist

Name :

Signature :

Peninah Torki



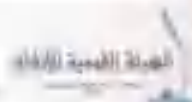
المعمل المركزي
شركة جمعه بدر نوح

Consultant Engineer

Name :

Signature :

[Signature]

	Electric Express Train - HSR				 
	From El Ain El Bokhna City To El Alamein - MATROUH				
	Section - 1 From FORA TO MARS MATROUH				
	From Station 524+000 To Station 525+000				

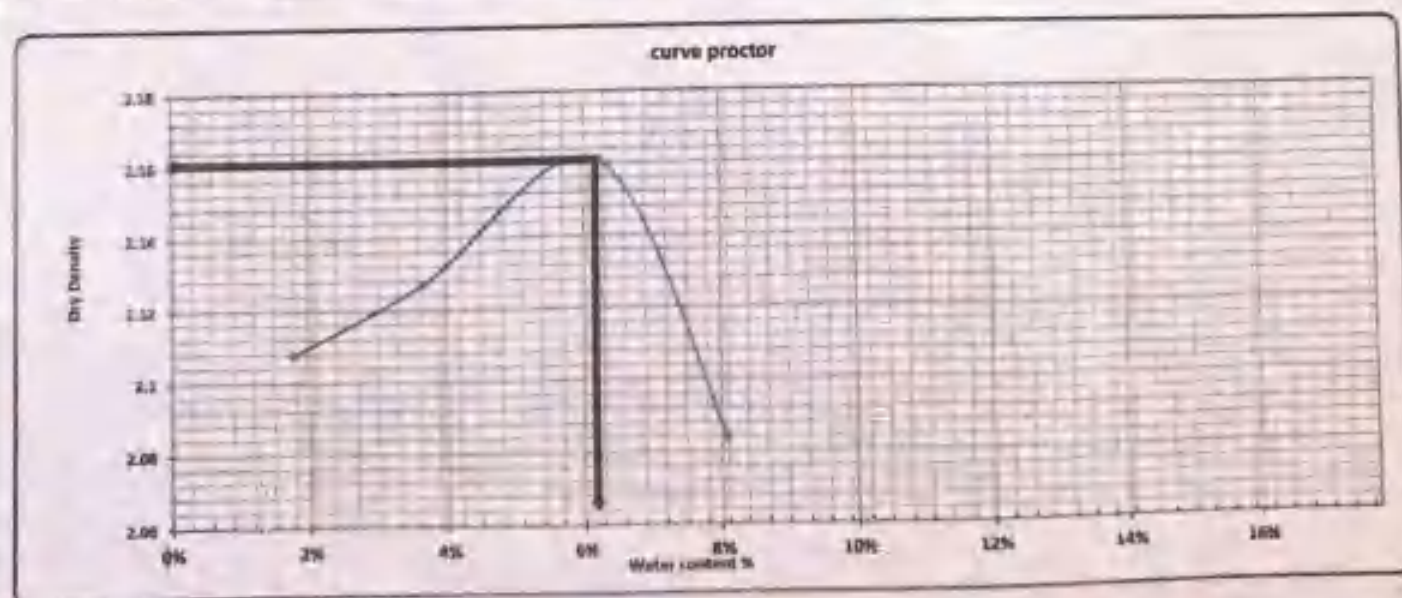
PROCTOR TEST

TESTING DATE	27/3/2023				
location	K.P524+800		ZONE	524+500	525+000
NAME COMPANY	AL Moustafa				

	operate by	GOMAA BADER LAB		
Weight of empty mold :	8536.8		MAX Dry Density	6.408
Mold Volume:	2104.9		Water content %	2.16

trial no :	1	2	3	4		
Wt. Of Mold+ wet soil	11068.6	11186.9	11276.8	11136		
WT. WET SOIL	4524.8	4659.8	4848.8	4758.8		
Wt. Density	2.149	2.209	2.299	2.257		

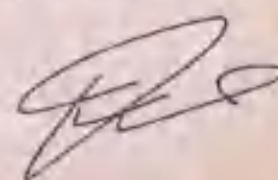
Tare No.	3	3	3	1	2	2	14	14			
Tare wt.	25.34	35.14	26.92	26.92	21.84	21.84	27	27			
Wt. Of wet soil & tare	148.35	146.35	158.66	158.66	149.34	149.34	121.85	121.85			
Wt. Of dry soil & tare	144	144	153.74	153.74	133.25	133.25	114.57	114.57			
Wt. Of water	2.3	2.3	4.9	4.9	7.1	7.1	7.3	7.3			
Wt. Of dry soil	118.9	118.9	126.8	126.8	118.4	118.4	87.6	87.6			
Water content %	2.0%	2.0%	3.9%	3.9%	6.4%	6.4%	8.3%	8.3%			
AV. Water content %	2.0%		3.9%		6.4%		8.3%				
Dry Density	2.108		2.127		2.161		2.084				



Contractor

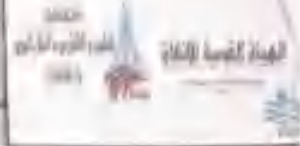
Consultant

المعمل المركزي
شركة جمعة بدر نوح





Electric Express Train - HSR
 From El Ain El Sokhna City To El Alamein - MATROUH
 Section - 7 From FORA To MARSA MATROUH
 From Station 554+000 To Station 568+177



PARTICLE SIZE DISTRIBUTION OF SOIL

TESTING DATE	26/3/2023	code			
location	K.P524+800	area		ZONE	524+500 525+000
NAME COMPANY	AL Moustafa				
1-visual inspection test	operate by	GOMAA BADER LAB			

2-Gradient test

A-gradation of bulk materials				SAMPLE WEIGHT (g)		33045.00	gm		table classify
sieve size	2	1.5	1	4/3	2/1	3/3	# 4	PASS	soil classify
Mass retained (g)	320.0	1890.0	3300.0	2000.0	5500.0	2620.0	4450.0		A-1-a
Cumulative Retained (g)	320.0	2160.0	5500.0	7600.0	13100.0	15810.0	20270.0	PRO	2.151
Cumulative Retained %	0.7	6.4	16.9	23.8	38.9	47.8	61.3	WC	6.4
Cumulative Passing %	99.3	93.6	83.1	77.0	60.1	52.2	38.7	CBR	55.0%

B-soft material gradation			WT.OF sample		600.00	gm
sieve size	10	40	200			
Cumulative Retained (g)	89.00	170.00	355.00			
Cumulative Retained %	13.90	34.00	67.00			
Cumulative Passing %	86.20	66.00	33.00			

C-General gradient										
sieve size(in)	2	1.5	1	3/4	1/2	3/8	# 4	# 10	# 40	# 200
sieve size(mm)	50.0	37.5	25.0	19.0	12.5	9.5	4.75	2.00	0.425	0.075
Cumulative Passing %	99.3	93.6	83.1	77.0	60.1	52.2	38.7	33.3	28.5	17.8

ملاحظات	ملاحظات	ملاحظات
N.P	N.P	N.P

Contractor

Consultant

المعمل المركزي
 شركة جمعة بدر نوح

Hassan

MATERIAL INSPECTION REQUEST

الهيئة العامة للإعانة



Contractor Company	AL-MOSTAFA COMPANY		Designer Company	K.K.																
Issued by Contractor	Name MOSTAFA THABET	Sign <i>Mostafa Thabet</i>	Date 11-04-2023	Time																
Received by ER			MIR	<table border="1"> <tr> <td>CT</td> <td>Q</td> <td>G</td> <td>DO</td> <td>MM</td> <td>YY</td> <td>HH</td> <td>MM</td> </tr> <tr> <td>5.2</td> <td>5.2</td> <td>5.2</td> <td>5.2</td> <td>04</td> <td>2023</td> <td></td> <td></td> </tr> </table>	CT	Q	G	DO	MM	YY	HH	MM	5.2	5.2	5.2	5.2	04	2023		
CT	Q	G	DO	MM	YY	HH	MM													
5.2	5.2	5.2	5.2	04	2023															

CODE-1	S1 to S21 Station Reference	D1 to S3 Depot Reference	Kp XXX Note For Kilometer point only Start Km is used
CODE-2	Work Activity		
CODE-3	Sub Element of Activity		

Description of Materials	PREPARED SUBGRADE MATERIAL RESULTS				
Location to be Used	From	To			
	524+780	525+000	P.SUB 1 (+0.25)		
	524+500	525+000	P.SUB 2 (+0.50)		
MAR Approval No				Date	
Supplier Name					
Test Requirement	Specification			Clause	
Reference Photos	Yes attached / No			Other	
Item	Description	Unit	Quantity	Arrival Date	Note
1	Sieve analysis	M3	5000	11-04-2023	
2	Classification	M3	5000	11-04-2023	
3	Proctor & O.M.C	M3	5000	11-04-2023	
4	LL & P.L & PI	M3	5000	11-04-2023	
5	C.B.R	M3	10000	11-04-2023	

Comments by:	Comments by:
A sample has been taken from fill material by K.K office to (AI NOUBI LAB) and the results founded meet the specifications and accepted.	

APPROVAL STATUS				
Organisation	Name	Sign	Date	A-AWC-R
Contractor	<i>Mostafa Thabet</i>	<i>Mostafa Thabet</i>		
QA/QC *	<i>Abdullah S.A.M.</i>	<i>Abdullah</i>		
GARB**				
Employers Representative				

* Designer

** Alignment / Bridges: Culvert Only

SUBMISSION of TEST RESULTS

الهيئة القومية للإسكان

الهيئة القومية للإسكان



Contractor Company	AL-MOSTAFA COMPANY			Designer Company	K.K					
Issued by Contractor	Name	Sign	Date	Time						
	Mostafa Thabet	<i>Mostafa Thabet</i>	11-04-2023							
Received by ER			MAR	C1 K.P 524	C2 E.W	C3 O.T	DD 11	MM 04	YY 2023	HH MM

CODE - 1	S1 to S21 Station Reference	D1 to S3 Depot Reference	Kp XXX Note For Kilometer point only Start Km is used
CODE - 2	Work Activity		
CODE - 3	Sub Element of Activity		

NB: Package 1 Only (Package 2 via Aconex)				
THE FOLLOWING TEST RESULTS ARE ATTACHED FOR REVIEW				
Description of Test Materials	Soil (A-1-a)			
Location of Test	K.P (524)			
Item	Specification	Test Requirement	Test Result Attachment	Remarks
1	ASTM D 75	Aggregate sampling	According to specification	
2	ASTM C 136	Sieve Analysis	According to specification	
3	ASTM D 1440	Passing sieve #200	8.30	
4	ASTM D 4318	Atterberg limit	N.P	
5	ASTM D 2974	Moisture content	6.50	
6	ASTM D 1557	Modified proctor	2.175	
7	ASTM D 1883	C.B.R	89.90	

Comments by:	Comments by:

APPROVAL STATUS				
Organisation	Name	Sign	Date	A-AWC-R
Contractor	<i>Mostafa Thabet</i>			A
Designer	<i>Youssef Ragab</i>	<i>Youssef Ragab</i>		A
GARB *				
Employers Representative				

* Alignment / Bridges: Culvert Only

 K.K. KUTUBIYAH & PARTNERS GENERAL CONTRACTING & TRADING (INCORPORATED IN EGYPT)	Electric Express Train - HSR From El Ain El Soshna City To El Alamein - MATROUH Section - 7 From FOKA To MARSA MATROUH From Station 594+000 To Station 599+177	الهيئة العامة للاستشارات العامة للتجارة والصناعة (EGYPT)

Operating Lab	AL Nuby Central Lab
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PARTICLE SIZE DISTRIBUTION OF SOIL

TESTING DATE:	11-4-2023	Code	Zone	524+500	525+000
LOCATION	K.P (524+800)	NO (P-1)			
NAME COMPANY	Al Mustafa				

1-visual inspection test

2-Gradient test

A-gradation of bulk materials				SAMPLE WEIGHT (g)		17419.00		gm	Table classify	
sieve size	2	1.5	1	4/3	2/1	3/3	# 4	PASS	Soil Classify	A-1-a
Mass retained (g)	122.0	1856.0	2489.0	812.0	981.0	1098.0	4079.0		PRO	2.175
Cumulative Retained (g)	122.0	1958.0	4457.0	5069.0	6030.0	7128.0	11207.0		WC	6.50
Cumulative Retained %	0.7	11.2	25.6	29.1	34.6	40.9	64.3		CBR	
Cumulative Passing %	99.3	88.8	74.4	70.9	65.4	59.1	35.7		Los Angeles	30.96

B-soft material gradation			WT.OF sample		800.00		gm
sieve size	10	40	200				
Cumulative Retained (g)	133.00	258.00	383.00				
Cumulative Retained %	26.60	51.60	76.60				
Cumulative Passing %	73.40	48.40	23.40				

C-General gradient										
sieve size(in)	2	1.5	1	3/4	1/2	3/8	# 4	# 10	# 40	# 200
sieve size(mm)	50.0	37.5	25.0	19.0	12.5	9.5	4.75	2.00	0.425	0.075
Cumulative Passing %	99.3	88.8	74.4	70.9	65.4	59.1	35.7	26.2	17.3	8.3

ATTERBERG LIMITS	LIQUID LIMIT (L.L.)	PLASTIC LIMIT (P.L.)	PLASTIC INDEX (P.I.)
	N.P	N.P	N.P

Contractor

Consultant



Youssef Ragab



Electric Express Train - HSR
 From El Ain El Sokhna City To El Alamein - MATROUH
 Section - 7 From FOKA TO MARSA MATROUH
 From Station 504+000 To Station 509+177



PROCTOR TEST

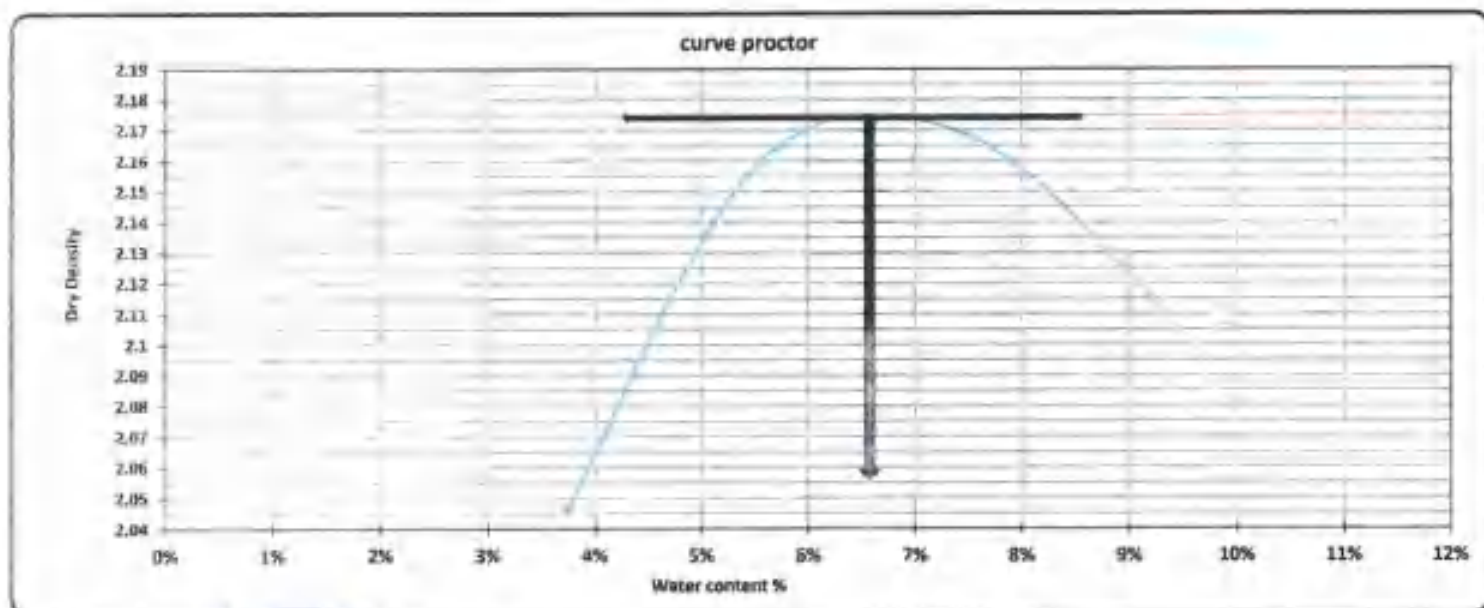
TESTING DATE:	11-4-2023	Code	zone	524++500	525+000
LOCATION	K.P (524+800)	MO (P-1)			
NAME COMPANY	Al Mustafa				

Weight of empty mold :	6037.0
Mold Volume:	2113.0




MAX Dry Density	2.175
Water content %	6.5

trial no :	1	2	3	4	
Wt. Of Mold+ wet soil	10523.0	10854.0	10962.0	10899	
WT. WET SOIL	4486.0	4817.0	4925.0	4862.0	
Wt. Density	2.123	2.280	2.331	2.301	

Tare No.	75	16	22	40	8	15	26	19		
Tare wt.	88	33.9	54.1	46.4	46.8	31.9	55	44.4		
Wt. Of wet soil & tare	150.0	150.0	150.0	150.0	150.0	150.0	150.0	150.0		
Wt. Of dry soil & tare	146.5	145.9	145.0	144.5	142.7	141.8	141.7	140.7		
Wt. Of water	3.5	4.1	5.0	5.5	7.3	8.2	8.3	9.3		
Wt. Of dry soil	91.5	112.0	90.9	98.1	95.9	109.9	86.7	96.3		
Water content %	3.8%	3.7%	5.5%	5.6%	7.6%	7.5%	9.6%	9.7%		
AV. Water content %	3.7%		5.6%		7.5%		9.6%			
Dry Density	2.046		2.160		2.167		2.099			



Consultant
 Youssef Ragab

 ENGINEERING CONSULTING OFFICE المكتب الاستشاري الهندسي أ.م. خالد شندوب	 الهيئة العامة للنقل	Electric Express Train - HSR	 المركز القومي للإحصاءات
		From El Ain El Bahria City To El Alamein - MATROUH Section - 7 From FOKA To MARSA MATROUH From Station 504+000 To Station 525+177	

TESTING DATE:	11-4-2023	code	zone	524+500	525+000
LOCATION	K.P (524+800+)	MO (P-1)			
NAME COMPANY	Al Mustafa				
	Los Anglos abrasion AASHTO-T96				

Results:-

Weight of sample before test (gm)	Weight of sample after test (gm)	Abrasion ratio (%)
5000	3452	30.96

Lab. Specialist

Name :

Sign :

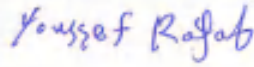
Lab. Engineer

Name : 

Sign :



Consultant Engineer

Name : 

Sign :

California Bearing Ratio TEST

Testing Date:	13-4-2023	Code:	FROM STA :	524+500	525+000
Location:	K.P (524+800)	MO (P-1)			
Company Name:	Al Mustafa				

Test Results

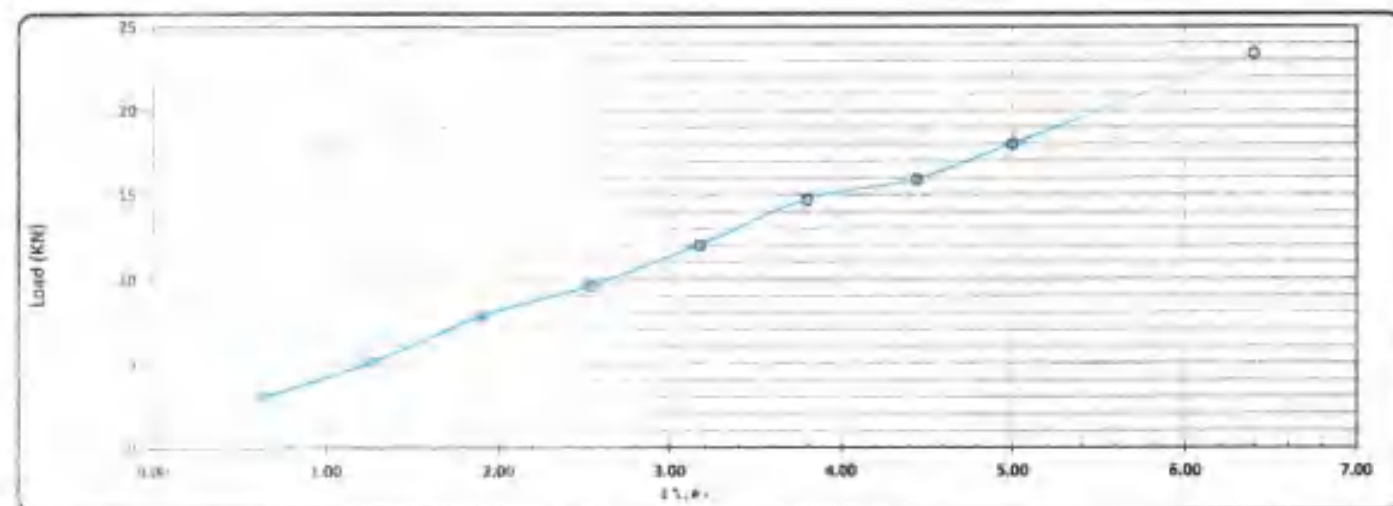
Compaction % for Mold	
Mold No.	3
Mold Vol. (cm ³)	2025
Mold Wt. (gm)	5034
Mold Wt. - Wat Wt. (gm)	9725
Wat Wt. (gm)	4691
Wat Density (g/cm ³)	2.317
Dry Density (g/cm ³)	2.175
Proctor Density (g/cm ³)	2.175
Compaction %	100.0

Moisture Ratio After Compacted Mold	
Tare No.	15
Tare WT. (gm)	31.9
Tare WT. + Wat WT. (gm)	150
Tare WT. + Dry WT. (gm)	142.8
Water WT. (gm)	7.2
Dry WT. (gm)	110.9
Moisture Content %	6.5

Swelling	
Mold No.	3
Date	13-4-2023
Initial Height (mm)	0.00
Final Height (mm)	0.00
Difference	0
Sample Height (mm)	120.00
Swelling Ratio %	0.0%

Loading Reading

46.50	0.60	1.27	1.91	2.54	3.18	3.80	4.45	5.00	6.40
Load Reading (mm)	0.10	0.17	0.26	0.32	0.40	0.49	0.53	0.60	0.78
Load (KN)	3.0	5.1	7.8	9.6	12.0	14.7	15.9	18.0	23.4



Calculations

Proctoration	Load	Standard Load	CBR	Mold - Compaction	Compaction	CBR
(mm)	(KN)	(lb)	(%)	(%)	(%)	% على نسبة 1A
2.50	9.60	13.4	71.9%	100	98	70.5%
4.00	18.00	20.0	89.9%			88.1%

Lab. Specialist

Name:

Sign:

Lab. Engineer

Name:

Sign:



Consultant Engineer

Name:

Sign:

Youssef Rafab

MATERIAL INSPECTION REQUEST

الهيئة العامة للإعطاء
General Authority for Grants



Contractor Company	AL-MOSTAFA COMPANY			Designer Company	K.K						
Issued by Contractor	Name	Sign	Date	Time							
	MOSTAFA THABET		10-09-2023								
Received by ER			MIR	C1	C2	C3	DD	MM	YY	HH	MM
				4.8	1.6	0.7	10	09	2023		

CODE-1	S1 to S21 Station Reference	D1 to S3 Depot Reference	Kp XXX Note For Kilometer point only Start Km is used
CODE-2	Work Activity		
CODE-3	Sub Element of Activity		

Description of Materials	SUB-BALLAST MATERIAL RESULTS					
Location to be Used	From	TO				
	524+500	525+000	SUB BALLAST 1 (+0.70)			
	524+500	525+000	SUB BALLAST 2 (+0.90)			
MAR Approval No				Date		
Supplier Name						
Test Requirement				Specification	Clause	
Reference Photos	Yes attached / No			Other		
Item	Description	Unit	Quantity	Arrival Date	Note	
1	Sieve analysis	M3	5000	10-09-2023		
2	Classification	M3	5000	10-09-2023		
3	Proctor & O.M.C	M3	5000	10-09-2023		
4	L.L & P.L & PI	M3	5000	10-09-2023		
5	C.B.R	M3	10000	10-09-2023		
Comments by:			Comments by:			
A sample has been taken from fill material by K.K office to (Al TAWAKOL LAB) and the results founded meet the specifications and accepted.						

APPROVAL STATUS				
Organisation	Name	Sign	Date	A-AWC-R
Contractor				
QA/QC *	Hassan			
GARB **				
Employers Representative				

* Designer

** Alignment / Bridges: Culvert Only

SUBMISSION of TEST RESULTS

الهيئة العامة للإنتاج
البناء



Contractor Company	AL-MOSTAFA COMPANY			Designer Company	K.K						
Issued by Contractor	Name	Sign	Date	Time							
	Mostafa Thabet	<i>مصطفى ثابت</i>	12-09-2023								
Received by ER			MAR	C1	C2	C3	DD	MM	YY	H	MM
				KCP 524	E-W	D.T	42	09	2023		



CODE-1	S1 to S21 Station Reference	D1 to S3 Depot Reference	Kp XXX Note For Kilometer point only Start Km is used
CODE-2	Work Activity		
CODE-3	Sub Element of Activity		

NB: Package 1 Only (Package 2 via Aconex)				
THE FOLLOWING TEST RESULTS ARE ATTACHED FOR REVIEW				
Description of Test Materials	Soil (A-1-a)			
Location of Test	K.P (524)			
Item	Specification	Test Requirement	Test Result Attachment	Remarks
1	ASTM D 75	Aggregate sampling	According to specification	
2	ASTM C 136	Sieve Analysis	According to specification	
3	ASTM D 1440	Passing sieve #200	4.65	
4	ASTM D 4318	Atterberg limit	N.P	
5	ASTM D 2974	Moisture content	7.10	
6	ASTM D 1557	Modified proctor	2.23	
7	ASTM D 1883	C.B.R	93.4	

Comments by:	Comments by:

APPROVAL STATUS				
Organisation	Name	Sign	Date	A-AWC-R
Contractor	<i>مصطفى ثابت</i>	<i>مصطفى ثابت</i>		A
Designer	Hassan	<i>for Hassan</i>	2023	A
GARB *				
Employers Representative				

* Alignment / Bridges: Culvert Only






Electric Express Train - HSR

From El Ain El Sokhna City To El Alamein - MATROUH

Section - 7 From FOKA To MARSA MATROUH

From Station 504+000 To Station 568+177

Operating lap

Al Tawkol Central Lab

PARTICLE SIZE DISTRIBUTION OF SOIL					
TESTING DATE:	10-09-2023	code	ZONE	524+500	525+000
LOCATION	K.P (524+750)	(mass) SUB BALLAST (1)	Material	SUB BALLAST	
NAME COMPANY	Al Mustafa		QUANTITY	5000 M	

visual inspection test

Gradient test

gradation of bulk materials				SAMPLE WEIGHT (gm)		41406.000		gm	table classify	
sieve size	2 "	1.5 "	1 "	3/4 "	1/2 "	3/8 "	# 4	PASS	soil classify	
Mass retained (g)	0.0	1254.0	4775.0	4523.0	6850.0	6960.0	5070.0	12054.0	CLASS	A-1-a
umulative Retained (g)	0.0	1254.0	6029.0	10552.0	17402.0	24362.0	29432.0		PRO	2.230
Cumulative Retained %	0.0	3.0	14.5	25.4	41.9	58.7	70.9		WC	7.1
Cumulative Passing %	100.0	97.0	85.5	74.6	58.1	41.3	29.00		CBR	93.40
									LA	25.4
									S.G	2.520

loft material gradation				WT.OF sample		500.00		gm
sieve size	#10	#40	#200					
umulative Retained (g)	150.00	320.00	420.00					
Cumulative Retained %	30.00	64.00	84.00					
umulative Passing %	70.00	36.00	16.00					

General gradient										
sieve size(in)	2 "	1.5 "	1 "	3/4 "	1/2 "	3/8 "	# 4	# 10	# 40	# 200
sieve size(mm)	50.0	37.5	25.0	19.0	12.5	9.5	4.75	2.00	0.425	0.075
umulative Passing %	100.0	85.00	85.5	74.60	50.1	41.3	29.1	20.3	10.5	4.60

ATTERBERG LIMTS	LIQUID LIMIT (L.L)	PLASTIC LIMIT (P.L.)	PLASTIC INDEX (P.I.)
	N.L	N.P	N.PI

Contractor




ENG AHMED HALEEM



Consultant

Hassan



 ENGINEERING CONSULTING OFFICE المكتب الاستشاري الهندسي ا.م. خالد فاضل	 شركة مصر للهندسة والبناء (Shouk El-Sayed & Co.) SYSTEM SHAKIR	Electric Express Train - HSR From El Ain El Sokhna City To El Alamein - MATROUH Section - 7 From FOKA To MARSA MATROUH From Station 504+000 To Station 568+177		 الهيئة العامة للحكومية (General Authority of State Railways)
		Absorbition & Aggregate specific gravity AASHTO-T85		

TESTING DATE:	10/09/2023	code	Station	524+500	525+000
LOCATION	K.P (524+750)	(mos) SUB BALLAST (1)	Material	SUB BALLAST	
NAME COMPANY	Al Mostafa		QUANTITY	5000 M	

Weight of sample	2500	gm
Weight of saturated surface dry sample (B)	2540	gm
Weight of saturated sample in water (C)	1553	gm
Weight of dry sample after heating (A)	2490	gm

Results:-

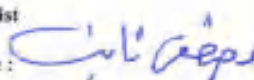
Bulk specific gravity = $A / (B-C)$	2.523	
Bulk specific gravity (S.S.D) = $B / (B-C)$	2.573	
Apparent specific gravity = $A / (A-C)$	2.657	
Absorbation = $(B-A)/A$	2.008	%


Los Anglos Abrasion AASHTO-T96

Results:-

Weight of sample before test (gm)	Weight of sample after test (gm)	Abrasion ratio (%)
5000	3730	25.40

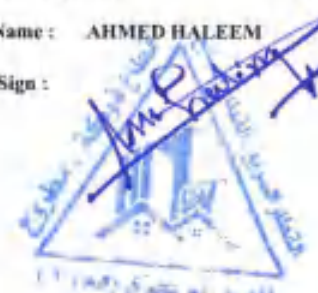
Lab. Specialist

Name : 

Sign : 

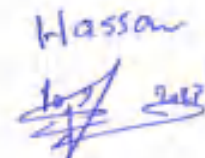
Lab. Engineer

Name : AHMED HALEEM

Sign : 

Consultant Engineer

Name : Hassan

Sign : 



Electric Express Train - HSR
From El Ain El Sokhna City To El Alamein - MATROUH
Section - 7 From FOKA TO MARSA MATROUH
From Station 504+000 To Station 566+177



MODIFIED PROCTOR TEST ASTM D-1557

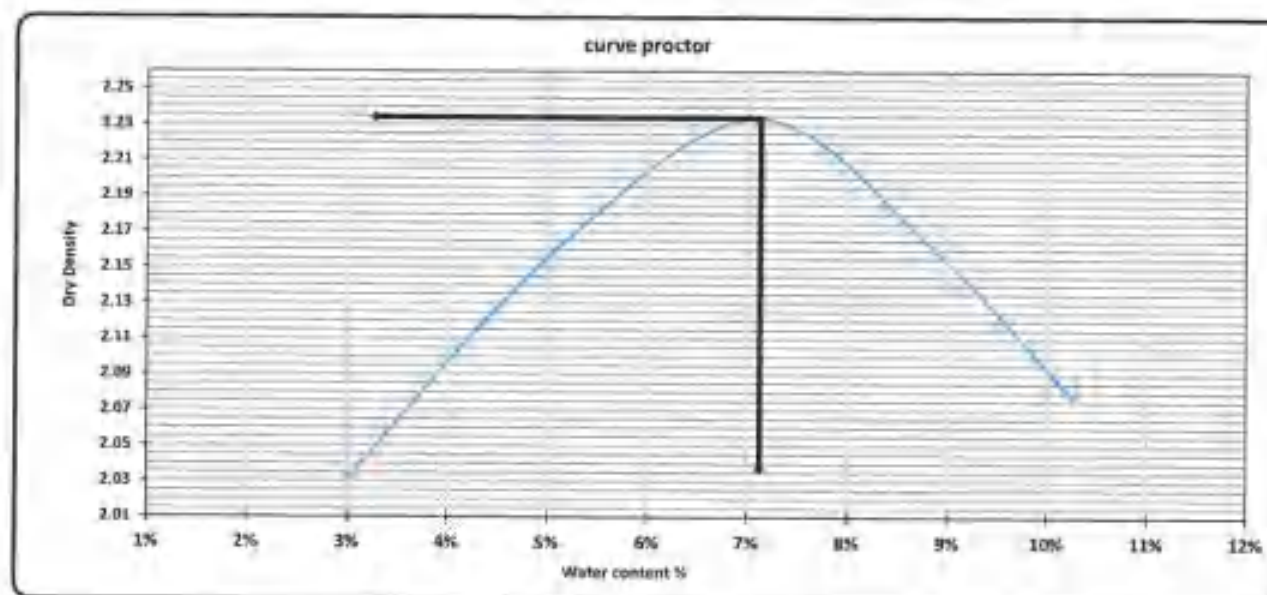
TESTING DATE:	11-08-2023	code:	ZONE	524+500	525+000
LOCATION	K.P (524+750)	Imma) SUB BALLAST (1)	Material	SUB BALLAST	
NAME COMPANY	Al Mostafa		QUANTITY	5000 M	

Weight of empty mold :	5620.0
Mold Volume:	2124.0

MAX Dry Density	2.233
Water content %	7.1%

trial no :	1	2	3	4	5
Wt. Of Mold+ wet soil	10070.0	10430.0	10700.0	10615	10485
WT. WET SOIL	4450.0	4810.0	5080.0	4905.0	4865.0
Wt. Density	2.095	2.265	2.392	2.352	2.290

Tare No.	10	11	1	2	3	4	5	6	7	8
Tare wt.	53.3	53.1	56.4	53.2	55.2	53.6	53.2	56.1	55.3	53.2
Wt. Of wet soil & tare	150.0	150.0	150.0	150.0	150.0	150.0	150.0	150.0	150.0	150.0
Wt. Of dry soil & tare	147.2	147.1	145.5	145.4	143.65	143.70	142.0	142.5	141.0	141.2
Wt. Of water	2.8	2.9	4.5	4.6	6.3	6.3	8.0	7.5	9.0	8.8
Wt. Of dry soil	93.9	94.0	89.1	92.2	88.5	90.1	88.8	86.4	85.7	88.0
Water content %	3.0%	3.1%	5.1%	5.0%	7.1%	7.0%	9.0%	8.7%	10.5%	10.0%
AV. Water content %	3.0%		5.0%		7.1%		8.9%		10.3%	
Dry Density	2.033		2.156		2.233		2.160		2.078	



Contractor

Consultant

California Bearing Ratio TEST

Testing Date :	12/9/2023	Code	FROM STA :	524+500	525+000
Location :	K.P (524+750)	(mos) SUB BALLAST(1)	: Material	SUB BALLAST	
Company Name	Al Mostafa		: Layer Thickness	500MM	

-: Test Results

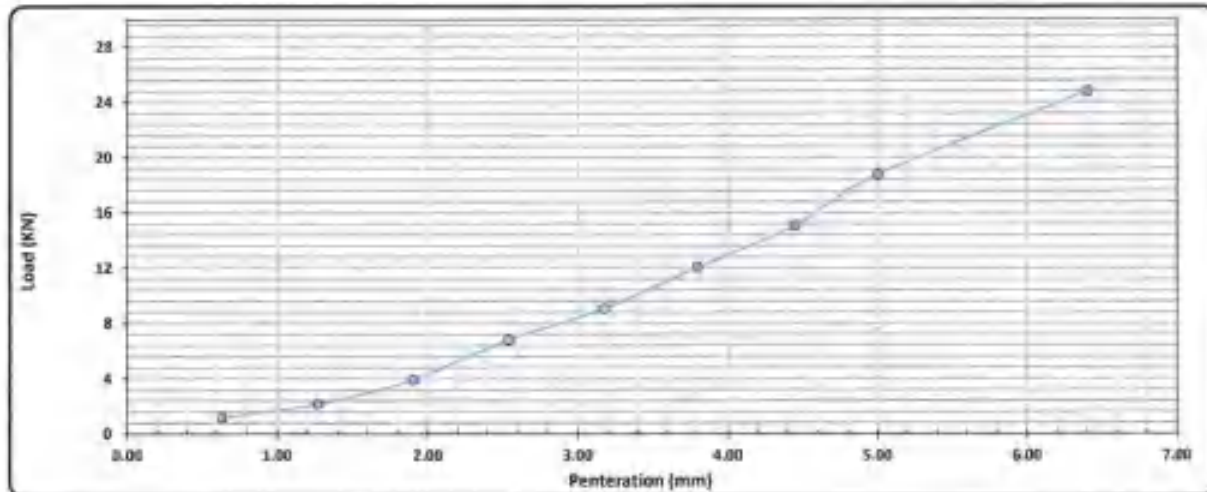
Compaction % for Mold	
Mold No.	1
Mold Vol.(cm ³)	2128
Mold WT. (gm)	5318
Mold WT. + Wet WT. (gm)	10385
Wet WT. (gm)	5075
Wet Density (g/cm ³)	2.394
Dry Density (g/cm ³)	2.238
Proctor Density (g/cm ³)	2.231
Compaction %	100.3

Moisture Ratio After Compacted Mold	
Tare No.	15
Tare WT. (gm)	55
Tare WT. + Wet WT. (gm)	150
Tare WT. + Dry WT. (gm)	143.8
Water WT. (gm)	6.2
Dry WT. (gm)	88.8
Moisture Content %	7.0

Swelling	
Mold No.	1
Date:	12/9/2023
Initial Height (mm)	8.80
Final Height (mm)	8.80
Difference	0.00
Sample Height (mm)	120
Swelling Ratio %	0.00%

Loading Reading :

Penetration (mm)	0.64	1.27	1.91	2.54	3.18	3.80	4.43	5.00	6.40
Load Reading (Kg)	130	240	435	785	1005	1340	1675	2085	2788
Load (KN)	1.2	2.2	3.9	6.8	9.0	12.1	15.1	18.8	24.8



Calculations :-

Penetration	Load	Standard Load	CBR	Mold - Compaction	Compaction	CBR
(mm)	(Kc)	(lb)	(%)	(%)	(%)	100 Load de %
2.50	6.80	13.4	50.9%	100.3	100	50.7%
5.00	18.77	20.0	93.7%			93.4%

Lab. Specialist:

Name: 
Sign: 

Lab. Engineer

Name: AHMED HALEEM
Sign: 


Consultant Engineer

Name: Hassan
Sign: 

Plate Load Test Results

Company Name

Al Mustafa

Location

524+820

To

524+920

Station

524+900

Test Date

21/3/2023

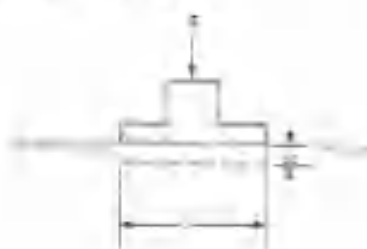
Layer level

-1.5

EQUIPMENT AND TEST PROCEDURE

The basis of the given equation is Boussinesq's theory of the relationship between the modulus of elasticity and the settlement of a circular rigid plate with the diameter D .

The load is applied to a circular rigid steel bearing plate by a hydraulic jack in several steps. The settlement under each load step is recorded. The following sketch shows the principle of the test.



P = load

Δs = settlement

D = diameter of the plate

The diameter D of the plate is generally 0.30 m. For very coarse grained material also plates with diameter $D = 0.60$ m and $D = 0.762$ m are used.

The load is applied in 6 load increments of equal size. Under each load step the settlement must come to a noticeable and (< 0.02 mm/minute). After the maximum load is reached the unloading procedure can begin. After that, the plate is reloaded in 5 steps. A loaded truck, an excavator or a roller usually serve as counterweight for the hydraulic jack.

Diameter = 300mm

Loading	Load	Load	Stress	Dist 1	Dist 2	Dist 3	Sett. 1	Sett. 2	Sett. 3	Avg. Sett.
Stage No.	Bar	kN	kN/m ²	mm	mm	mm	mm	mm	mm	mm
0.000	0.0	0.000	0.00	14.92	13.45		0.000	0.000		0.000
1.000	2.4	0.707	0.01	14.80	13.40		0.120	0.050		0.085
2.000	18.8	5.652	0.08	14.40	13.15		0.520	0.300		0.410
3.000	37.7	11.304	0.16	14.02	12.95		0.900	0.500		0.700
4.000	58.9	17.663	0.25	13.80	12.85		1.120	0.600		0.860
5.000	77.7	23.315	0.33	13.55	12.70		1.370	0.750		1.060
6.000	98.9	29.673	0.42	13.35	12.60		1.570	0.850		1.210
7.000	117.8	35.325	0.50	13.10	12.50		1.820	0.950		1.385
8.000	58.9	17.663	0.25	13.15	12.58		1.770	0.870		1.320
9.000	29.4	8.831	0.12	13.25	12.65		1.670	0.800		1.235
9.000	2.4	0.707	0.01	13.85	13.02		1.070	0.430		0.750
10.000	2.4	0.707	0.01	13.85	13.02		1.070	0.430		0.750
11.000	18.8	5.652	0.08	13.60	12.85		1.320	0.600		0.960
12.000	37.7	11.304	0.16	13.40	12.75		1.520	0.700		1.110
13.000	58.9	17.663	0.25	13.30	12.70		1.620	0.750		1.185
14.000	77.7	23.315	0.33	13.15	12.65		1.770	0.800		1.285
15.000	98.9	29.673	0.42	13.00	12.55		1.920	0.900		1.410

		σ	Δs	$\Delta \sigma$
0.7 σ_1	0.35	1.05688	0.39313	0.2
0.3 σ_1	0.15	0.66375		
0.7 σ_2	0.35	1.31278	0.14776	0.2
0.3 σ_2	0.15	1.17002		
D (mm)	300			
E_{v1}	114.47			
E_{v2}	315.22			
Area (kg/m ²)	0.87005			

E_{v2}/E_{v1}	2.75		
-----------------	------	--	--

$$E_v = 0.75 \cdot D \cdot \Delta \sigma / \Delta s$$

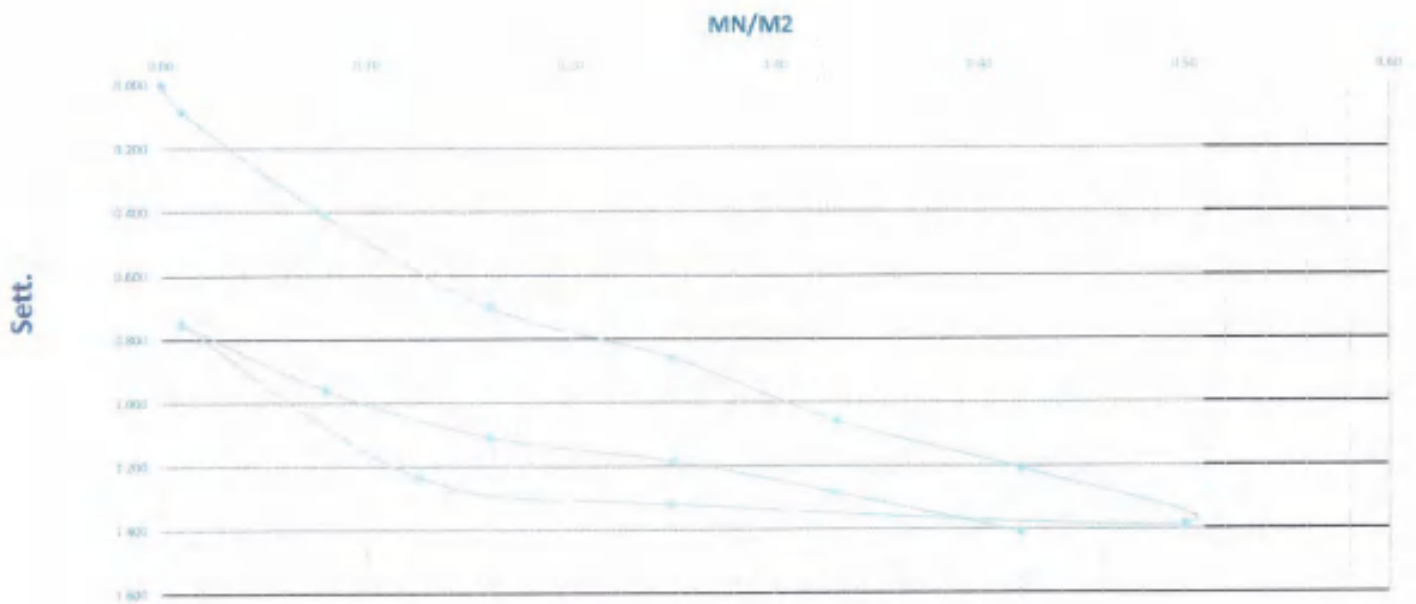
E_v = deformation modulus

$\Delta \sigma$ = load increment

Δs = settlement increment

D = diameter of the plate, generally 0.30 m

For this calculation $\Delta\sigma$ and Δs are usually taken from the load span between $0.3 \sigma_{max}$ and $0.7 \sigma_{max}$.



Lab. Specialist

Name :

Sign :

Lab. Engineer - out

Name :

Sign :



Consultant Engineer

Name :

Hassan

Sign :

Plate Load Test Results

Company Name

المصطفى

Location

524+800

To

524+920

Station

524+880

Test Date

11-04-2023

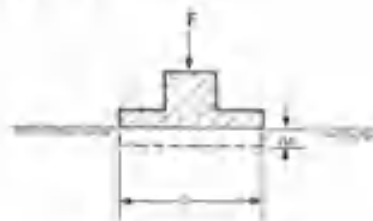
Layer level

ferma

EQUIPMENT AND TEST PROCEDURE :-

The basis of the given equation is Boussinesq's theory of the relationship between the modulus of elasticity and the settlement of a circular rigid plate with the diameter D .

The load is applied to a circular rigid steel bearing plate by a hydraulic jack in several steps. The settlement under each load step is recorded. The following sketch shows the principle of the test.



F = load

Δs = settlement

D = diameter of the plate

The diameter D of the plate is generally 0.30 m. For very coarse grained material also plates with diameter $D = 0.60$ m and $D = 0.762$ m are used.

The load is applied in 6 load increments of equal size. Under each load step the settlement must come to a noticeable end (< 0.02 mm/minute). After the maximum load is reached the unloading procedure can begin. After that, the plate is reloaded in 5 steps. A loaded truck, an excavator or a roller usually serve as counterweight for the hydraulic jack.

Diameter = 300mm

Loading	Load	Load	Stress	Dial 1	Dial 2	Dial 3	Sett. 1	Sett. 2	Sett. 3	Avg. Sett.
Stage No.	Bar	kN	kN/M ²	mm	mm	mm	mm	mm	mm	mm
0.000	0.0	0.000	0.00	20.00	20.00		0.000	0.000		0.000
1.000	2.1	0.707	0.01	19.94	19.90		0.060	0.100		0.080
2.000	17.1	5.652	0.08	19.82	19.80		0.180	0.200		0.190
3.000	34.2	11.304	0.16	19.55	19.72		0.450	0.280		0.365
4.000	53.3	17.663	0.25	19.36	19.61		0.640	0.390		0.515
5.000	70.5	23.315	0.33	19.19	19.52		0.810	0.480		0.645
6.000	89.8	29.673	0.42	18.96	19.40		1.040	0.600		0.820
7.000	106.8	35.325	0.50	18.76	19.31		1.240	0.690		0.965
8.000	53.4	17.663	0.25	18.84	19.38		1.160	0.620		0.890
9.000	26.7	8.831	0.12	18.94	19.49		1.060	0.510		0.785
9.000	2.1	0.707	0.01	19.18	19.60		0.820	0.400		0.610
10.000	2.1	0.707	0.01	19.18	19.60		0.820	0.400		0.610
11.000	17.1	5.652	0.08	19.14	19.56		0.860	0.440		0.650
12.000	34.2	11.304	0.16	19.05	19.50		0.950	0.500		0.725
13.000	53.3	17.663	0.25	18.95	19.46		1.050	0.540		0.795
14.000	70.5	23.315	0.33	18.88	19.41		1.120	0.590		0.855
15.000	89.8	29.673	0.42	18.78	19.36		1.220	0.640		0.930

σ	ϵ	AS	Δs
0.7 σ_1	0.35	0.69313	0.35
0.3 σ_1	0.15	0.34313	
0.7 σ_2	0.35	0.87167	0.18166
0.3 σ_2	0.15	0.69	
D (mm)	300		
E_{s1}	128.57		
E_{s2}	247.71		
Area (sqm)	0.07065		

E_s/E_{s1}	1.93		
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$$E_s = 0.79 \cdot D \cdot \Delta s / \Delta s$$

E_s = deformation modulus

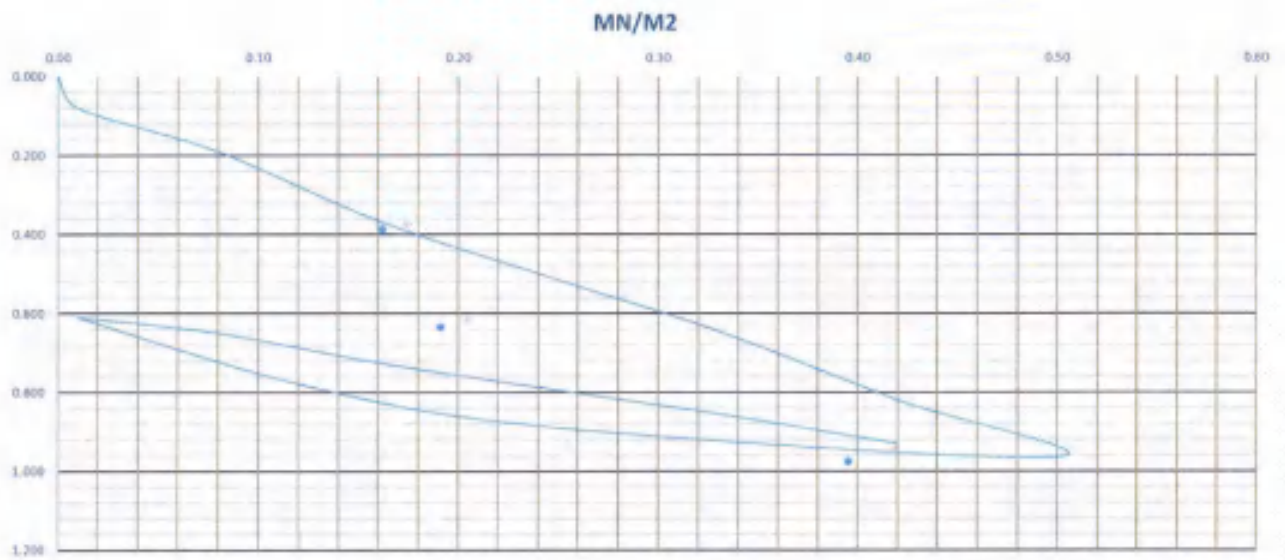
Δs = load increment

D_s = settlement increment

D = diameter of the plate, generally 0.30 m

For this calculation $\Delta\sigma$ and Δs are usually taken from the load span between $0.3 \sigma_{max}$ and $0.7 \sigma_{max}$.

Sett.



Lab. Specialist

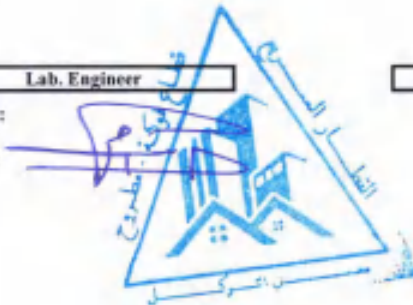
Name :

Sign :

Lab. Engineer

Name :

Sign :



Consultant Engineer

Name :

Sign :

Youssef Ragab

Plate Load Test Results

Company Name

Location

Test Date

Layer level

المصطفى

524+800

11-04-2023

ferma

To

524+920

Station

524+840

EQUIPMENT AND TEST PROCEDURE :-

The basis of the given equation is Boussinesq's theory of the relationship between the modulus of elasticity and the settlement of a circular rigid plate with the diameter D .

The load is applied to a circular rigid steel bearing plate by a hydraulic jack in several steps. The settlement under each load step is recorded. The following sketch shows the principle of the test.



P = load

s = settlement

D = diameter of the plate

The diameter D of the plate is generally 0.30 m. For very coarse grained material also plates with diameter $D = 0.60$ m and $D = 0.762$ m are used.

The load is applied in 8 load increments of equal size. Under each load step the settlement must come to a noticeable end (≤ 0.02 mm/minute). After the maximum load is reached the unloading procedure can begin. After that, the plate is reloaded in 5 steps. A loaded truck, an excavator or a roller usually serve as counterweight for the hydraulic jack.

Diameter = 300mm

Loading	Load	Load	Stress	Dial 1	Dial 2	Dial 3	Sett. 1	Sett. 2	Sett. 3	Avg. Sett.
Stage No.	Bar	kN	MPa	mm	mm	mm	mm	mm	mm	mm
0.000	0.0	0.000	0.00	20.00	20.00		0.000	0.000		0.000
1.000	2.1	0.707	0.01	19.94	19.96		0.060	0.040		0.050
2.000	17.1	5.652	0.08	19.81	19.88		0.190	0.120		0.155
3.000	34.2	11.304	0.16	19.61	19.81		0.390	0.190		0.290
4.000	53.3	17.663	0.25	19.35	19.72		0.650	0.280		0.465
5.000	70.5	23.315	0.33	19.20	19.68		0.800	0.320		0.560
6.000	89.8	29.673	0.42	19.05	19.62		0.950	0.380		0.665
7.000	106.8	35.325	0.50	18.90	19.58		1.100	0.420		0.760
8.000	53.4	17.663	0.25	18.95	19.63		1.050	0.370		0.710
9.000	26.7	8.831	0.12	19.06	19.69		0.940	0.310		0.625
9.000	2.1	0.707	0.01	19.28	19.79		0.720	0.210		0.465
10.000	2.1	0.707	0.01	19.28	19.79		0.720	0.210		0.465
11.000	17.1	5.652	0.08	19.24	19.77		0.760	0.230		0.493
12.000	34.2	11.304	0.16	19.16	19.76		0.840	0.240		0.540
13.000	53.3	17.663	0.25	19.10	19.72		0.900	0.280		0.590
14.000	70.5	23.315	0.33	19.05	19.66		0.950	0.340		0.645
15.000	89.8	29.673	0.42	18.98	19.60		1.020	0.400		0.710

σ	ΔS	$\Delta \sigma$
0.7 σ_1	0.35	0.58187
0.5 σ_1	0.15	0.27313
0.7 σ_2	0.35	0.65944
0.5 σ_2	0.15	0.525
D (mm)	300	
E_s	145.75	
E_{s1}	334.72	
Area (sq.m)	0.07065	

E_{s2}/E_{s1}	2.30	
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$$E_s = 0.75 \cdot D \cdot \Delta \sigma / \Delta s$$

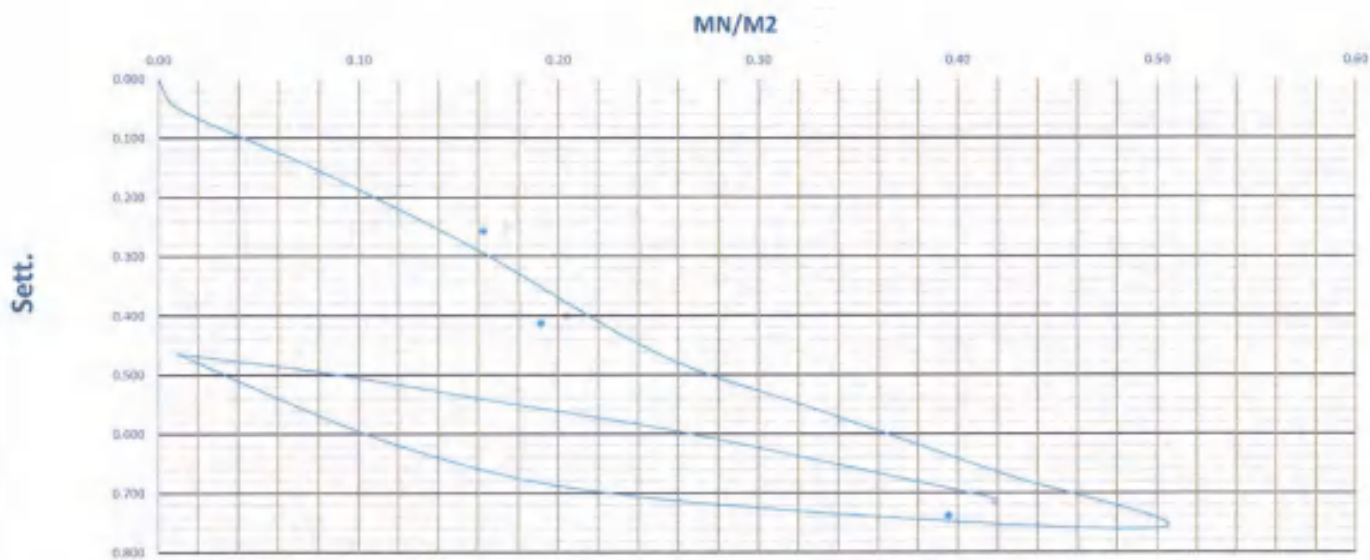
E_s = deformation modulus

$\Delta \sigma$ = load increment

Δs = settlement increment

D = diameter of the plate, generally 0.30 m

For this calculation $\Delta\sigma$ and Δs are usually taken from the load span between $0.3 \sigma_{max}$ and $0.7 \sigma_{max}$.



Lab. Specialist

Name :

Sign :

Lab. Engineer

Name :

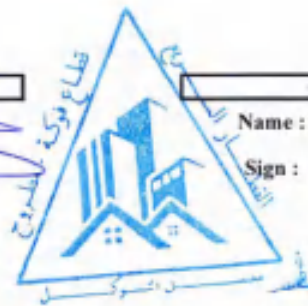
Sign :

Consultant Engineer

Name :

Sign :

Youssef Ragab



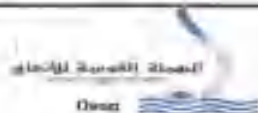


Plate Load Test Results

Company Name

المصطفى

Location

524+820

To

524+900

Station

524+880

Test Date

16-04-2023

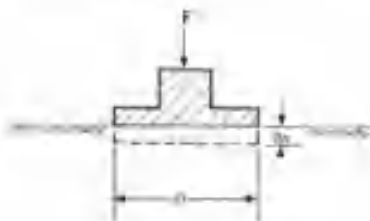
Layer level

P.S.G + 0.5

EQUIPMENT AND TEST PROCEDURE :-

The basis of the given equation is Boussinesq's theory of the relationship between the modulus of elasticity and the settlement of a circular rigid plate with the diameter D .

The load is applied to a circular rigid steel bearing plate by a hydraulic jack in several steps. The settlement under each load step is recorded. The following sketch shows the principle of the test.



$F = \text{load}$

$\Delta s = \text{settlement}$

$D = \text{diameter of the plate}$

The diameter D of the plate is generally 0.30 m. For very coarse grained material also plates with diameter $D = 0.60$ m and $D = 0.762$ m are used.

The load is applied in 6 load increments of equal size. Under each load step the settlement must come to a noticeable end (< 0.02 mm/minute). After the maximum load is reached the unloading procedure can begin. After that, the plate is reloaded in 5 steps. A loaded truck, an excavator or a roller usually serve as counterweight for the hydraulic jack.

Diameter = 300mm

Loadmg	Load	Load	Stress	Dist.1	Dist.2	Dist.3	Sett.1	Sett.2	Sett.3	Log. Sett.
Gauge No.	Bar	kg	N/m ²	mm	mm	mm	mm	mm	mm	mm
0.000	0.0	0.000	0.00	20.00	20.00		0.000	0.000		0.000
1.000	2.1	0.707	0.01	19.95	19.87		0.050	0.130		0.090
2.000	17.1	5.652	0.08	19.55	19.61		0.450	0.390		0.420
0.080	34.2	11.304	0.16	19.10	19.40		0.900	0.600		0.750
4.000	53.3	17.663	0.25	18.85	19.10		1.150	0.900		1.025
5.000	70.5	23.315	0.33	18.66	18.90		1.340	1.020		1.180
6.000	89.8	29.673	0.42	18.46	18.82		1.540	1.180		1.360
7.000	106.8	35.325	0.50	18.25	18.66		1.750	1.340		1.545
8.000	53.4	17.663	0.25	18.33	18.74		1.670	1.260		1.465
9.000	26.7	8.831	0.12	18.42	18.82		1.580	1.180		1.380
9.000	2.1	0.707	0.01	18.69	19.03		1.310	0.970		1.140
10.000	2.1	0.707	0.01	18.69	19.03		1.310	0.970		1.140
11.000	17.1	5.652	0.08	18.64	19.00		1.360	1.000		1.180
12.000	34.2	11.304	0.16	18.58	18.93		1.450	1.070		1.260
13.000	53.3	17.663	0.25	18.44	18.85		1.560	1.150		1.355
14.000	70.5	23.315	0.33	18.36	18.77		1.640	1.230		1.435
15.000	89.8	29.673	0.42	18.26	18.70		1.740	1.300		1.520

		Δ	AS	Δs
0.7 σ_0	0.35	1.19813	0.48937	0.2
0.3 σ_0	0.15	0.79875		
0.7 σ_0	0.35	1.45389	0.23388	0.2
0.3 σ_0	0.15	1.22		
D (mm)	300			
E_{s1}	91.92			
E_{s2}	192.40			
Area (Sqmm)	0.07065			

E_{s1}/E_{s2}	1.96		
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$$E_s = 0.75 \cdot D \cdot \Delta s / A_s$$

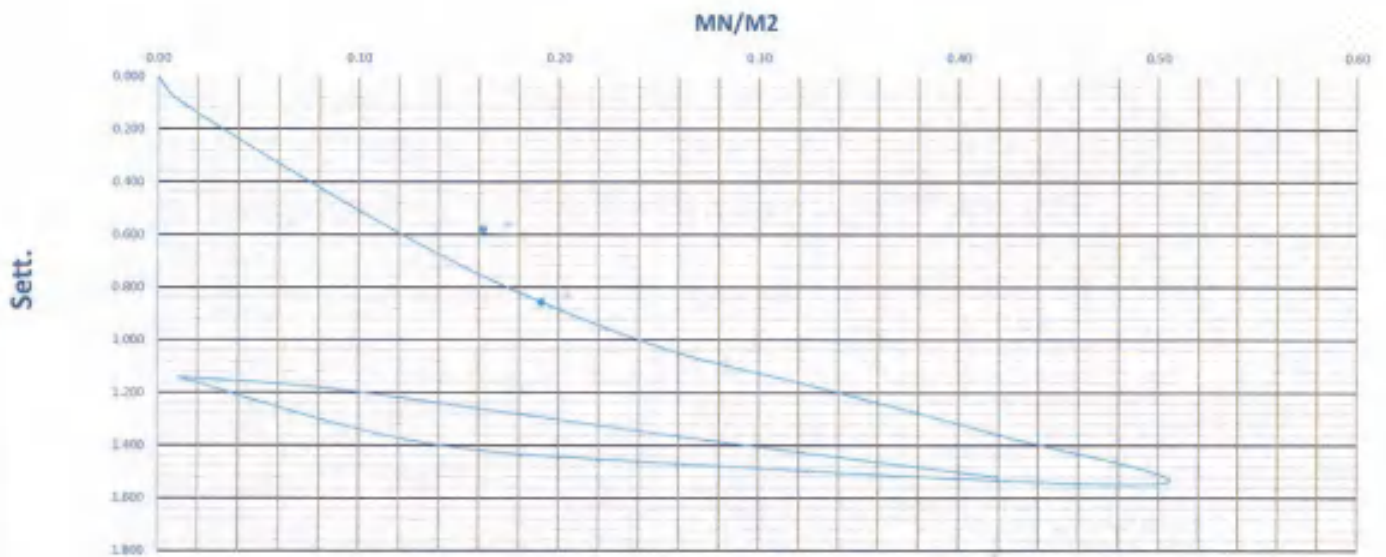
E_s = deformation modulus

D_s = load increment

D_s = settlement increment

D = diameter of the plate, generally 0.30 m

For this calculation $\Delta\sigma$ and Δs are usually taken from the load span between $0.3 \sigma_{max}$ and $0.7 \sigma_{max}$.



Lab. Specialist

Name :

Sign :

Lab. Engineer

Name :

Sign :

Consultant Engineer

Name :

Sign :

17/7/2020

Plate Load Test Results

Company Name

المصطفى

Location

524+820

To

524+900

Status

524/838

Test Date

16-04-2023

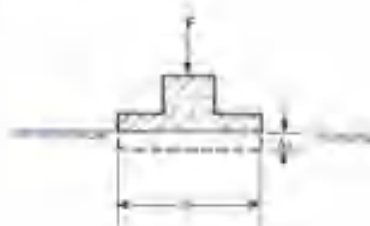
Layer level

P.S.G + 0.5

EQUIPMENT AND TEST PROCEDURE :-

The basis of the given equation is Boussinesq's theory of the relationship between the modulus of elasticity and the settlement of a circular rigid plate with the diameter D .

The load is applied to a circular rigid steel bearing plate by a hydraulic jack in several steps. The settlement under each load step is recorded. The following sketch shows the principle of the test.



$F = \text{Load}$

$\Delta s = \text{settlement}$

$D = \text{diameter of the plate}$

The diameter D of the plate is generally 0.30 m. For very coarse grained material also plates with diameter $D = 0.60$ m and $D = 0.762$ m are used.

The load is applied in 5 load increments of equal size. Under each load step the settlement must come to a noticeable and < 0.02 mm/minute. After the maximum load is reached the unloading procedure can begin. After that, the plate is reloaded in 5 steps. A loaded truck, an excavator or a roller usually serve as counterweight for the hydraulic jack.

Diameter = 300mm

Loading	Load	Load	Stress	Dial 1	Dial 2	Dial 3	Sett. 1	Sett. 2	Sett. 3	App. Sett.
Stage No.	Bar	KN	KN/m ²	mm	mm	mm	mm	mm	mm	mm
0.000	0.0	0.000	0.00	20.00	20.00		0.000	0.000		0.000
1.000	2.1	0.707	0.01	19.75	19.95		0.250	0.050		0.150
2.000	17.1	5.652	0.08	19.15	19.65		0.850	0.350		0.600
3.000	34.2	11.304	0.16	18.75	19.40		1.250	0.600		0.925
4.000	53.3	17.663	0.25	18.42	19.15		1.580	0.850		1.215
5.000	70.5	23.315	0.33	18.15	19.03		1.850	0.970		1.410
6.000	89.8	29.673	0.42	17.92	18.85		2.080	1.150		1.615
7.000	106.8	35.325	0.50	17.68	18.71		2.320	1.290		1.805
8.000	53.4	17.663	0.25	17.75	18.80		2.250	1.200		1.725
9.000	26.7	8.831	0.12	17.85	18.90		2.150	1.100		1.625
9.000	2.1	0.707	0.01	18.15	19.12		1.850	0.880		1.365
10.000	2.1	0.707	0.01	18.15	19.12		1.850	0.880		1.365
11.000	17.1	5.652	0.08	18.07	19.06		1.930	0.940		1.435
12.000	34.2	11.304	0.16	18.02	19.00		1.980	1.000		1.490
13.000	53.3	17.663	0.25	17.87	18.91		2.130	1.090		1.610
14.000	70.5	23.315	0.33	17.77	18.85		2.230	1.150		1.690
15.000	89.8	29.673	0.42	17.65	18.78		2.350	1.220		1.785

		σ	ΔS	$\Delta \sigma$
$0.7 \sigma_1$	0.35	1.44875	0.56437	0.2
$0.3 \sigma_1$	0.15	0.58438		
$0.7 \sigma_2$	0.35	1.71111	0.2061	0.2
$0.3 \sigma_2$	0.15	1.50261		
D (mm)	300			
E_{s1}	79.73			
E_{s2}	218.34			
Area / Sq.m	0.07065			

E_s/E_{s1}	2.74		
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$$E_s = 0.75 \times D \times \Delta \sigma / \Delta s$$

E_s = deformation modulus

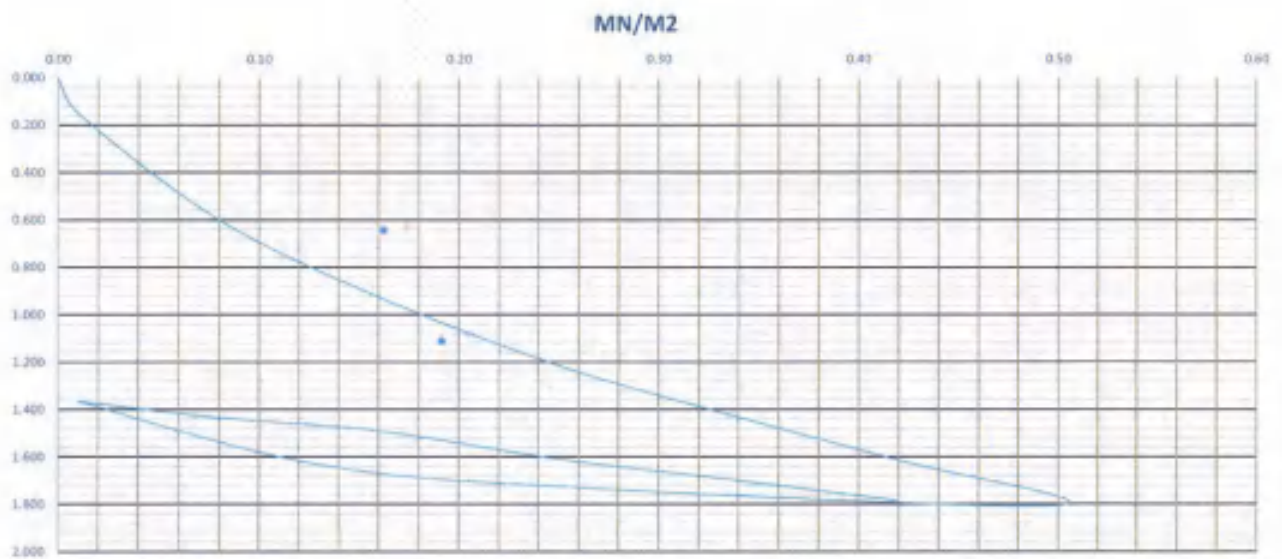
D_s = load increment

D_s = settlement increment

D = diameter of the plate, generally 0.30 m

For this calculation $\Delta\sigma$ and Δs are usually taken from the load span between $0.3 \sigma_{max}$ and $0.7 \sigma_{max}$.

Sett.



Lab. Specialist

Name :

Sign :

Lab. Engineer

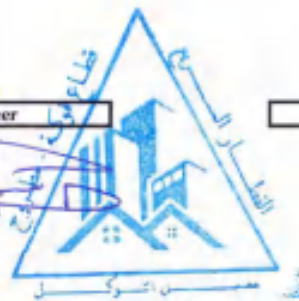
Name :

Sign :

Consultant Engineer

Name :

Sign :



17/7/2023

Plate Load Test Results

Company Name

المصطفى

Location

524+820

To

524+900

Station

524+860

Test Date

16-04-2023

Layer level

P.S.G + 0.5

EQUIPMENT AND TEST PROCEDURE :-

The basis of the given equation is Boussinesq's theory of the relationship between the modulus of elasticity and the settlement of a circular rigid plate with the diameter D .

The load is applied to a circular rigid steel bearing plate by a hydraulic jack in several steps. The settlement under each load step is recorded. The following sketch shows the principle of the test.



P = load

s = settlement

D = diameter of the plate

The diameter D of the plate is generally 0.30 m. For very coarse grained material also plates with diameter $D = 0.60$ m and $D = 0.762$ m are used.

The load is applied in 8 load increments of equal size. Under each load step the settlement must come to a noticeable end (≤ 0.02 mm/minute). After the maximum load is reached the unloading procedure can begin. After that, the plate is reloaded in 5 steps. A loaded truck, an excavator or a roller usually serve as counterweight for the hydraulic jack.

Diameter = 300mm

Loading	Load	Load	Stress	Dial 1	Dial 2	Dial 3	Sett. 1	Sett. 2	Sett. 3	Avg. Sett.
Stage No.	Bar	KN	N/K/M2	mm	mm	mm	mm	mm	mm	mm
0.000	0.0	0.000	0.00	20.00	20.00		0.000	0.000		0.000
1.000	2.1	0.707	0.01	19.88	19.90		0.120	0.100		0.110
2.000	17.1	5.652	0.08	19.40	19.65		0.600	0.350		0.475
3.000	34.2	11.304	0.16	19.15	19.30		0.850	0.700		0.775
4.000	53.3	17.663	0.25	18.82	19.12		1.180	0.880		1.030
5.000	70.5	23.315	0.33	18.70	18.90		1.300	1.100		1.200
6.000	89.8	29.673	0.42	18.42	18.70		1.580	1.300		1.440
7.000	106.8	35.325	0.50	18.30	18.60		1.700	1.400		1.550
8.000	53.4	17.663	0.25	18.40	18.70		1.600	1.300		1.450
9.000	26.7	8.831	0.12	18.60	18.80		1.400	1.200		1.300
9.000	2.1	0.707	0.01	18.70	19.00		1.300	1.000		1.150
10.000	2.1	0.707	0.01	18.70	19.00		1.300	1.000		1.150
11.000	17.1	5.652	0.08	18.65	18.95		1.350	1.050		1.200
12.000	34.2	11.304	0.16	18.60	18.90		1.400	1.100		1.250
13.000	53.3	17.663	0.25	18.45	18.80		1.550	1.200		1.375
14.000	70.5	23.315	0.33	18.40	18.75		1.600	1.250		1.425
15.000	89.8	29.673	0.42	18.32	18.62		1.680	1.380		1.530

		α	AS	IS
0.7 σ_1	0.35	1.34375	0.60625	0.2
0.3 σ_1	0.15	0.7375		
0.7 σ_2	0.35	1.44833	0.19833	0.2
0.3 σ_2	0.15	1.25000		
D (mm)	300			
E_{s1}	74.21			
E_{s2}	226.90			
Area (sq.m)	0.07065			

E_{s1}/E_{s2}	3.96		
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$$E_s = 8.73 \cdot D \cdot \Delta \sigma / \Delta s$$

E_s = deformation modulus

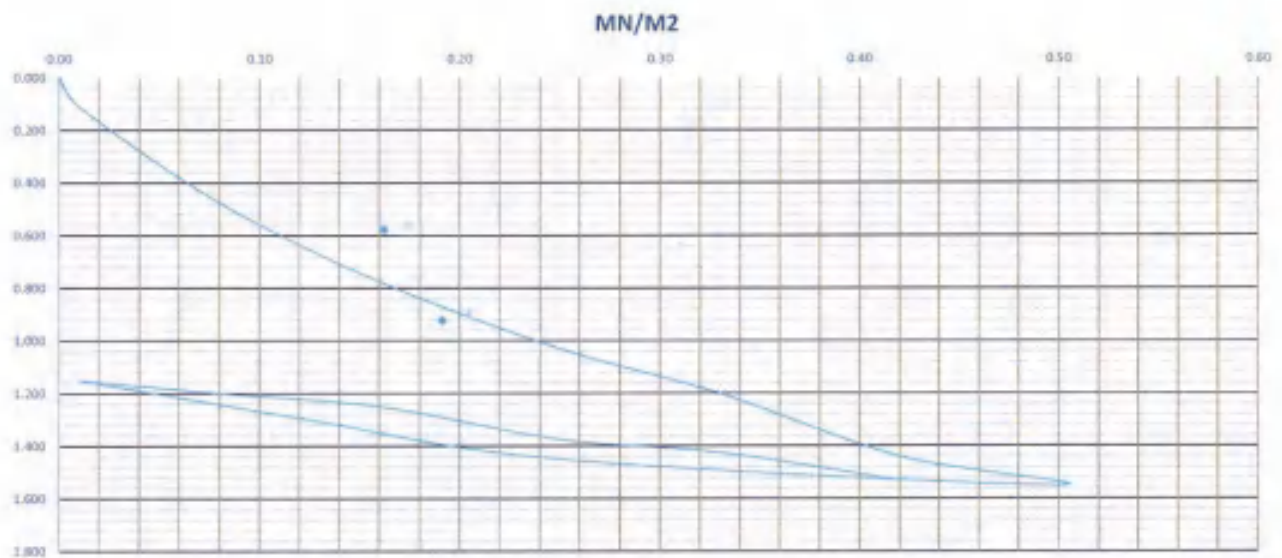
$\Delta \sigma$ = load increment

Δs = settlement increment

D = diameter of the plate, generally 0.30 m

For this calculation $\Delta\sigma$ and Δs are usually taken from the load span between $0.3 \sigma_{max}$ and $0.7 \sigma_{max}$.

Sett.



Lab. Specialist

Name :

Sign :

Lab. Engineer

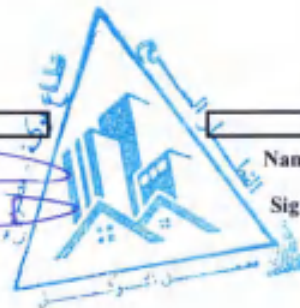
Name :

Sign :

Consultant Engineer

Name :

Sign :



Signature
11/7/2020

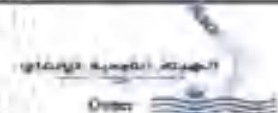


Plate Load Test Results

Company Name

AL MOSTAFA

Location

524+500

To

524+580

Station

524+520

Test Date

9-09-2023

Layer level

P.S.G +0.50

EQUIPMENT AND TEST PROCEDURE :-

The basis of the given equation is Boussinesq's theory of the relationship between the modulus of elasticity and the settlement of a circular rigid plate with the diameter D .

The load is applied to a circular rigid steel bearing plate by a hydraulic jack in several steps. The settlement under each load step is recorded. The following sketch shows the principle of the test.



F = load

Δs = settlement

D = diameter of the plate

The diameter D of the plate is generally 0.30 m. For very coarse grained material also plates with diameter $D = 0.60$ m and $D = 0.762$ m are used.

The load is applied in 6 load increments of equal size. Under each load step the settlement must come to a noticeable end (< 0.02 mm/minute). After the maximum load is reached the unloading procedure can begin. After that, the plate is reloaded in 5 steps. A loaded truck, an excavator or a roller usually serve as counterweight for the hydraulic jack.

Diameter = 300mm

Test No.	Load	Load	Stress	Dist 1	Dist 2	Dist 3	Sett. 1	Sett. 2	Sett. 3	Avg. Sett.
Stage No.	Bar	KN	MPa	mm	mm	mm	mm	mm	mm	mm
0.000	0.0	0.000	0.00	20.00	20.00		0.000	0.000		0.000
1.000	2.1	0.707	0.01	19.80	19.75		0.200	0.250		0.225
2.000	17.1	5.652	0.08	19.60	19.55		0.400	0.450		0.425
3.000	34.2	11.304	0.16	19.25	19.26		0.750	0.740		0.745
4.000	53.3	17.663	0.25	18.80	19.05		1.200	0.950		1.075
5.000	70.5	23.315	0.33	18.55	18.95		1.450	1.050		1.250
6.000	89.8	29.673	0.42	18.40	18.80		1.600	1.200		1.400
7.000	106.8	35.325	0.50	18.05	18.72		1.950	1.280		1.615
8.000	53.4	17.663	0.25	18.10	18.76		1.900	1.240		1.570
9.000	26.7	8.831	0.12	18.18	18.89		1.820	1.110		1.465
9.000	2.1	0.707	0.01	18.31	19.00		1.690	1.000		1.345
10.000	2.1	0.707	0.01	18.31	19.00		1.690	1.000		1.345
11.000	17.1	5.652	0.08	18.50	18.99		1.700	1.010		1.355
12.000	34.2	11.304	0.16	18.22	18.95		1.780	1.050		1.415
13.000	53.3	17.663	0.25	18.16	18.86		1.840	1.140		1.490
14.000	70.5	23.315	0.33	18.14	18.80		1.860	1.200		1.530
15.000	89.8	29.673	0.42	18.06	18.71		1.940	1.290		1.615

		α	AS	Δs
$0.7 \sigma_1$	0.35	1.21168	0.50688	0.2
$0.3 \sigma_2$	0.15	0.705		
$0.7 \sigma_2$	0.35	1.54889	0.18389	0.2
$0.3 \sigma_3$	0.15	1.365		
D (mm)	300			
E_{v1}	89.78			
E_{v2}	244.71			
Area (Sq.m)	0.07065			

E_{v2}/E_{v1}	2.72		
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$$E_v = 0.75 \cdot D \cdot \Delta s / \Delta s$$

E_v = deformation modulus

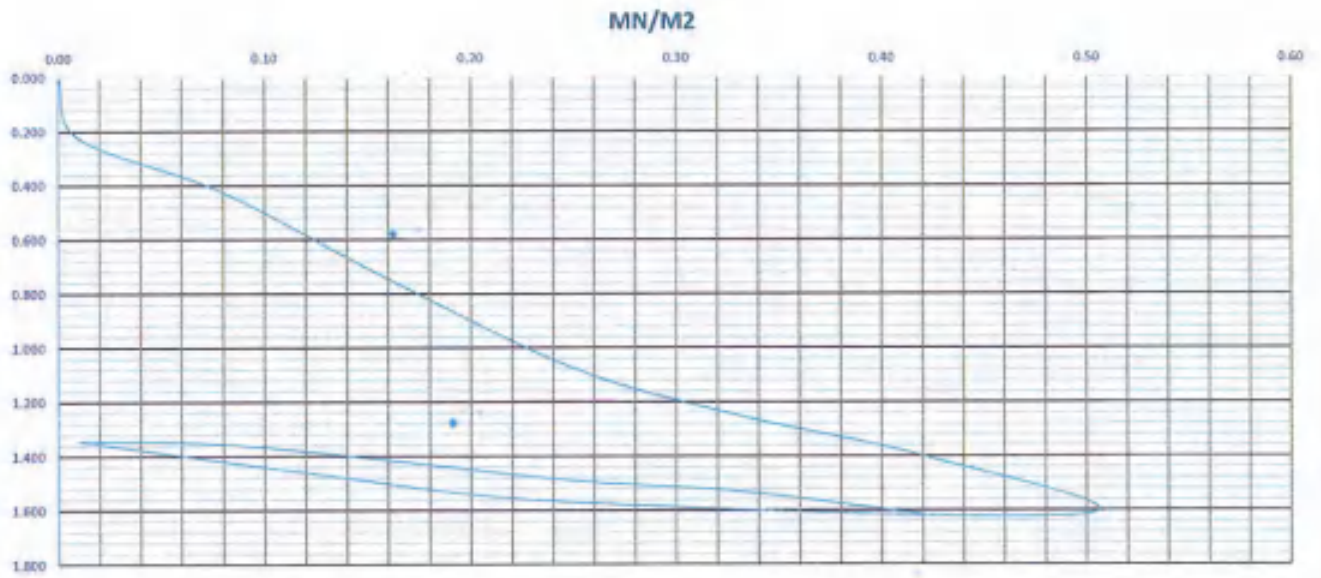
Δs = load increment

Δs = settlement increment

D = diameter of the plate, generally 0.30 m

For this calculation $\Delta\sigma$ and Δs are usually taken from the load span between $0.3 \sigma_{max}$ and $0.7 \sigma_{max}$.

Sett.



Lab. Specialist

Name :

Sign :

Lab. Engineer

Name :

Sign :



Consultant Engineer

Name :

Sign :

[Signature]



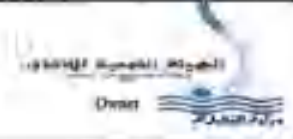
Owner Consultant



Contractor Consultant



Contractor



Owner

Plate Load Test Results

Company Name

AL MOSTAFA

Location

524+500

To

524+580

Station

524+540

Test Date

9-09-2023

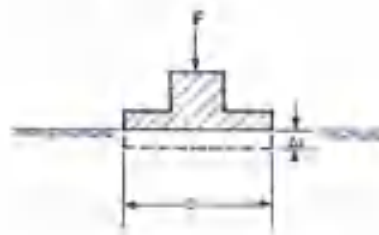
Layer level

P.S.G +0.50

EQUIPMENT AND TEST PROCEDURE :-

The basis of the given equation is Boussinesq's theory of the relationship between the modulus of elasticity and the settlement of a circular rigid plate with the diameter D .

The load is applied to a circular rigid steel bearing plate by a hydraulic jack in several steps. The settlement under each load step is recorded. The following sketch shows the principle of the test.



F = load

Δs = settlement

D = diameter of test plate

The diameter D of the plate is generally 0.30 m. For very coarse grained material also plates with diameter $D \approx 0.60$ m and $D \approx 0.762$ m are used.

The load is applied in 6 load increments of equal size. Under each load step the settlement must come to a noticeable end (< 0.02 mm/minute). After the maximum load is reached the unloading procedure can begin. After that, the plate is reloaded in 5 steps. A loaded truck, an excavator or a roller usually serve as counterweight for the hydraulic jack.

Diameter = 300mm

Loading	Load	Load	Settle	Dist 1	Dist 2	Dist 3	Sett. 1	Sett. 2	Sett. 3	Avg. Sett.
Stage No.	Bar	KN	KN/M2	mm	mm	mm	mm	mm	mm	mm
0.000	0.0	0.000	0.00	20.00	20.00		0.000	0.000		0.000
1.000	2.1	0.707	0.01	19.85	19.79		0.150	0.210		0.180
2.000	17.1	5.652	0.08	19.63	19.57		0.370	0.430		0.400
3.000	34.2	11.304	0.16	19.27	19.28		0.730	0.720		0.725
4.000	53.3	17.663	0.25	18.85	19.06		1.150	0.940		1.045
5.000	70.5	23.315	0.33	18.50	18.99		1.500	1.010		1.255
6.000	89.8	29.673	0.42	18.35	18.85		1.650	1.150		1.400
7.000	106.8	35.325	0.50	18.07	18.70		1.930	1.300		1.615
8.000	53.4	17.663	0.25	18.12	18.76		1.880	1.240		1.560
9.000	26.7	8.831	0.12	18.17	18.90		1.830	1.100		1.465
9.000	2.1	0.707	0.01	18.33	19.02		1.670	0.980		1.325
10.000	2.1	0.707	0.01	18.33	19.02		1.670	0.980		1.325
11.000	17.1	5.652	0.08	18.32	18.09		1.680	1.010		1.345
12.000	34.2	11.304	0.16	18.25	18.93		1.750	1.070		1.410
13.000	53.3	17.663	0.25	18.17	18.84		1.830	1.160		1.495
14.000	70.5	23.315	0.33	18.10	18.80		1.900	1.200		1.550
15.000	89.8	29.673	0.42	18.05	18.70		1.950	1.300		1.625

		σ	ΔS	$\Delta \sigma$
0.7 n_1	0.35	1.23188	0.5275	0.1
0.3 σ_1	0.15	0.68438		
0.7 σ_2	0.35	1.56667	0.20166	0.2
0.3 σ_2	0.15	1.365		
D (mm)	300			
E_{s1}	85.21			
E_{s2}	222.34			
Area (Sq.m)	0.07065			

$F \pm \Delta F$	$(\Delta \sigma)$		
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$$E_s = 0.75 \cdot D \cdot \Delta \sigma / \Delta s$$

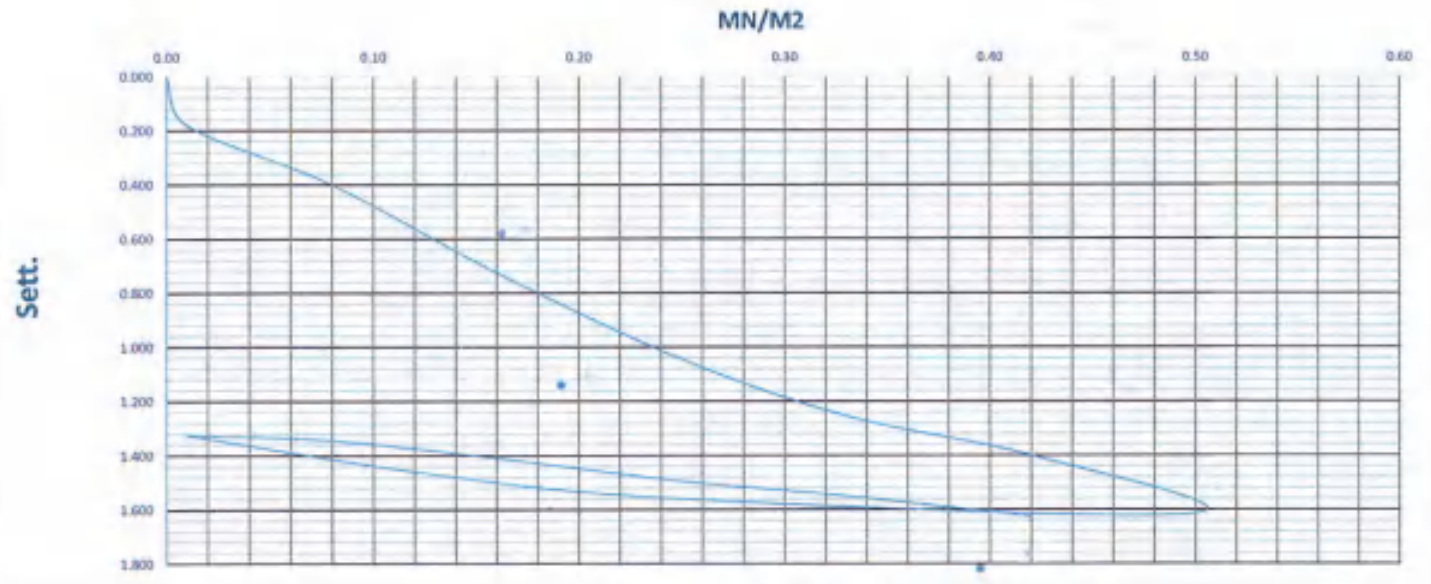
E_s = deformation modulus

$\Delta \sigma$ = load increment

Δs = settlement increment

D = diameter of the plate, generally 0.30 m

For this calculation $\Delta\sigma$ and Δs are usually taken from the load span between $0.3 \sigma_{max}$ and $0.7 \sigma_{max}$.



Lab. Specialist

Name :

Sign :

Lab. Engineer

Name :

Sign :

Consultant Engineer

Name :

Sign : *Abdullah*

Plate Load Test Results

Company Name

AL MOSTAFA

Location

524+500

To

524+580

Station

524+560

Test Date

9-09-2023

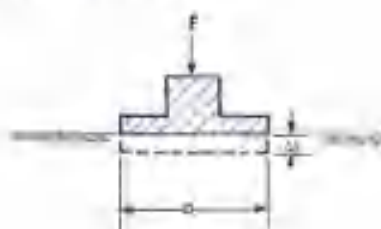
Layer level

P.S.G +0.50

EQUIPMENT AND TEST PROCEDURE :-

The basis of the given equation is Boussinesq's theory of the relationship between the modulus of elasticity and the settlement of a circular rigid plate with the diameter D .

The load is applied to a circular rigid steel bearing plate by a hydraulic jack in several steps. The settlement under each load step is recorded. The following sketch shows the principle of the test.



F = Load

a = settlement

D = diameter of the plate

The diameter D of the plate is generally 0.30 m. For very coarse grained material also plates with diameter $D = 0.60$ m and $D = 0.762$ m are used.

The load is applied in 5 load increments of equal size. Under each load step the settlement must come to a nilivable and (< 0.02 mm/minute). After the maximum load is reached the unloading procedure can begin. After that, the plate is reloaded in 5 steps. A loaded truck, an excavator or a roller usually serve as counterweight for the hydraulic jack.

Diameter = 300mm

Loading	Load	Load	Stress	Dial 1	Dial 2	Dial 3	Sett. 1	Sett. 2	Sett. 3	Avg. Sett.
Stage No.	Bar	kN	kN/m ²	mm	mm	mm	mm	mm	mm	mm
0.000	0.0	0.000	0.00	20.00	20.00		0.000	0.000		0.000
1.000	2.1	0.707	0.01	19.87	19.80		0.130	0.200		0.165
2.000	17.1	5.652	0.08	19.50	19.59		0.500	0.410		0.455
3.000	34.2	11.304	0.16	19.20	19.32		0.800	0.680		0.740
4.000	53.3	17.663	0.25	18.80	19.08		1.200	0.920		1.060
5.000	70.5	23.315	0.33	18.51	18.93		1.490	1.070		1.280
6.000	89.8	29.673	0.42	18.25	18.87		1.750	1.130		1.440
7.000	106.8	35.325	0.50	18.04	18.62		1.960	1.380		1.670
8.000	53.4	17.663	0.25	18.12	18.77		1.800	1.230		1.555
9.000	26.7	8.831	0.12	18.17	18.93		1.830	1.070		1.450
9.000	2.1	0.707	0.01	18.35	19.05		1.650	0.950		1.300
10.000	2.1	0.707	0.01	18.35	19.05		1.650	0.950		1.300
11.000	17.1	5.652	0.08	18.33	18.99		1.670	1.010		1.340
12.000	34.2	11.304	0.16	18.22	18.95		1.780	1.050		1.415
13.000	53.3	17.663	0.25	18.18	18.83		1.820	1.170		1.495
14.000	70.5	23.315	0.33	18.13	18.78		1.870	1.220		1.545
15.000	89.8	29.673	0.42	18.08	18.70		1.920	1.300		1.610

		k	AS	AS
0.7 σ_1	0.35	1.23875	0.51437	0.2
0.3 σ_1	0.15	0.70438		
0.7 σ_2	0.35	1.55944	0.17944	0.2
0.3 σ_2	0.15	1.38		
D (mm)	300			
E_{v1}	84.21			
E_{v2}	250.78			
Area (Sq.m)	0.07065			

E_{v2}/E_{v1}	2.98		
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$$E_v = 0.78 \cdot D \cdot \sigma_2 / \Delta s$$

E_v = deformation modulus

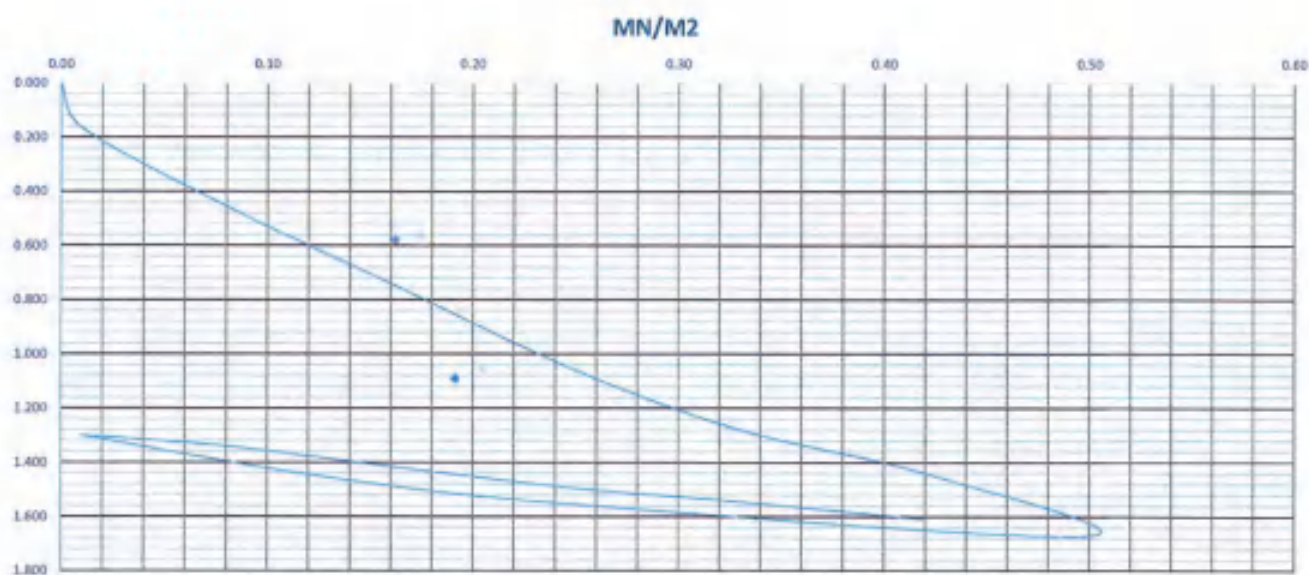
Δs = load increment

Δs = settlement increment

D = diameter of the plate, generally 0.30 m

For this calculation $\Delta\sigma$ and Δs are usually taken from the load span between $0.3 \sigma_{max}$ and $0.7 \sigma_{max}$.

Sett.



Lab. Specialist

Name :

Sign :

Lab. Engineer

Name :

Sign :



Consultant Engineer

Name :

Sign :

Plate Load Test Results

Company Name AL MOSTAFA

Location 524+580 To 524+660

Station 524+585

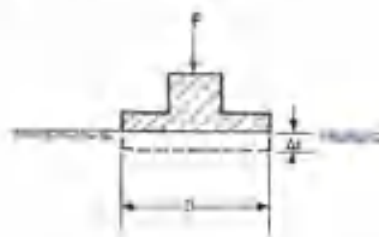
Test Date 9-09-2023

Layer level P.S.G +0.50

EQUIPMENT AND TEST PROCEDURE :-

The basis of the given equation is Boussinesq's theory of the relationship between the modulus of elasticity and the settlement of a circular rigid plate with the diameter D .

The load is applied to a circular rigid steel bearing plate by a hydraulic jack in several steps. The settlement under each load step is recorded. The following sketch shows the principle of the test.



P = load

Δs = settlement

D = diameter of the plate

The diameter D of the plate is generally 0.30 m. For very coarse grained material also plates with diameter $D = 0.60$ m and $D = 0.762$ m are used.

The load is applied in 6 load increments of equal size. Under each load step the settlement must come to a noticeable end (< 0.02 mm/minute). After the maximum load is reached the unloading procedure can begin. After that, the plate is reloaded in 5 steps. A loaded truck, an excavator or a roller usually serve as counterweight for the hydraulic jack.

Diameter = 300mm

Loading	Load	Load	Stress	Dial 1	Dial 2	Dial 3	Sett. 1	Sett. 2	Sett. 3	Avg. Sett.
Stage No.	Bar	KN	KN/M2	mm	mm	mm	mm	mm	mm	mm
0.000	0.0	0.000	0.00	20.00	20.00		0.000	0.000		0.000
1.000	2.1	0.707	0.01	19.85	19.76		0.150	0.240		0.195
2.000	17.1	5.652	0.08	19.62	19.52		0.380	0.480		0.430
3.000	34.2	11.304	0.16	19.34	19.29		0.660	0.710		0.685
4.000	53.3	17.663	0.25	19.00	19.09		1.000	0.910		0.955
5.000	70.5	23.315	0.33	18.89	18.93		1.110	1.070		1.090
6.000	89.8	29.673	0.42	18.55	18.86		1.450	1.140		1.295
7.000	106.8	35.325	0.50	18.44	18.70		1.560	1.390		1.430
8.000	53.4	17.663	0.25	18.49	18.82		1.510	1.180		1.345
9.000	26.7	8.831	0.12	18.56	18.92		1.440	1.080		1.260
9.000	2.1	0.707	0.01	18.69	19.09		1.310	0.910		1.110
10.000	2.1	0.707	0.01	18.69	19.09		1.310	0.910		1.110
11.000	17.1	5.652	0.08	18.67	19.05		1.330	0.950		1.140
12.000	34.2	11.304	0.16	18.62	19.01		1.380	0.990		1.185
13.000	53.3	17.663	0.25	18.55	18.92		1.450	1.080		1.265
14.000	70.5	23.315	0.33	18.51	18.84		1.490	1.160		1.325
15.000	89.8	29.673	0.42	18.48	18.72		1.520	1.280		1.400

		s	AS	Δs
0.7 σ_1	0.35	1.17688	0.52375	0.2
0.3 σ_1	0.15	0.65313		
0.7 σ_2	0.35	1.34167	0.17168	0.2
0.3 σ_2	0.15	1.17		
D (mm)	300			
E_{s1}	48.92			
E_{s2}	362.14			
Area (sq.m)	0.07065			

$E = 2E_s / 1$	3.65		
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$$E_s = 0.7P / D \cdot \Delta s / \Delta s$$

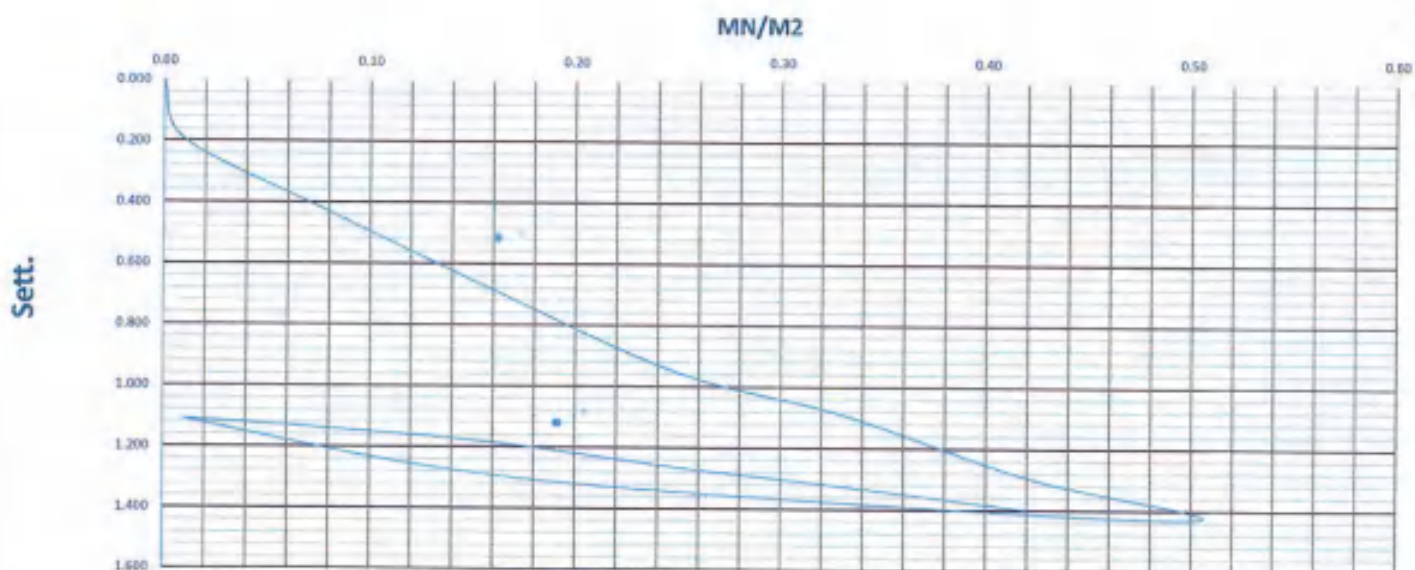
E_s = deformation modulus

Δs = load increment

D_s = settlement increment

D = diameter of the plate, generally 0.30 m

For this calculation $\Delta\sigma$ and Δs are usually taken from the load span between $0.3 \sigma_{max}$ and $0.7 \sigma_{max}$.



Lab. Specialist

Name :

Sign :

Lab. Engineer

Name :

Sign :



Consultant Engineer

Name :

Sign :



Plate Load Test Results

Company Name

AL MOSTAFA

Location

524+580

To

524+660

Station

524+600

Test Date

9-09-2023

Layer level

P.S.G +0.50

EQUIPMENT AND TEST PROCEDURE :-

The basis of the given equation is Boussinesq's theory of the relationship between the modulus of elasticity and the settlement of a circular rigid plate with the diameter D .

The load is applied to a circular rigid steel bearing plate by a hydraulic jack in several steps. The settlement under each load step is recorded. The following sketch shows the principle of the test.



P = Load
 Δs = settlement
 D = diameter of the plate

The diameter D of the plate is generally 0.30 m. For very coarse grained material also plates with diameter $D = 0.60$ m and $D = 0.762$ m are used.

The load is applied in 6 load increments of equal size. Under each load step the settlement must come to a noticeable end (< 0.02 mm/minute). After the maximum load is reached the unloading procedure can begin. After that, the plate is reloaded in 5 steps. A loaded truck, an excavator or a roller usually serve as counterweight for the hydraulic jack.

Diameter = 300mm

Loading	Load	Load	Stress	Dial 1	Dial 2	Dial 3	Sett 1	Sett 2	Sett 3	Acc. Sett.
Stage No.	Bar	kN	kN/m ²	mm	mm	mm	mm	mm	mm	mm
0.000	0.0	0.000	0.00	20.00	20.00		0.000	0.000		0.000
1.000	2.1	0.707	0.01	19.85	19.78		0.150	0.220		0.185
2.000	17.1	5.652	0.08	19.61	19.55		0.390	0.450		0.420
3.000	34.2	11.304	0.16	19.33	19.32		0.670	0.680		0.675
4.000	51.3	17.663	0.25	19.02	19.12		0.980	0.880		0.930
5.000	70.5	23.315	0.33	18.92	18.95		1.080	1.050		1.065
6.000	89.8	29.673	0.42	18.57	18.87		1.430	1.130		1.280
7.000	106.8	35.325	0.50	18.45	18.71		1.550	1.290		1.420
8.000	51.3	17.663	0.25	18.49	18.83		1.510	1.170		1.340
9.000	26.7	8.831	0.12	18.57	18.95		1.430	1.050		1.240
9.000	2.1	0.707	0.01	18.70	19.12		1.300	0.880		1.090
10.000	2.1	0.707	0.01	18.70	19.12		1.300	0.880		1.090
11.000	17.1	5.652	0.08	18.67	19.07		1.330	0.930		1.130
12.000	34.2	11.304	0.16	18.63	19.00		1.370	1.000		1.185
13.000	51.3	17.663	0.25	18.50	18.93		1.500	1.070		1.285
14.000	70.5	23.315	0.33	18.47	18.85		1.530	1.150		1.340
15.000	89.8	29.673	0.42	18.42	18.77		1.580	1.230		1.405

		σ	Δs	$\Delta \sigma$
0.7 σ_1	0.35	1.1575	0.51437	0.2
0.3 σ_2	0.15	0.64313		
0.7 σ_2	0.35	1.35444	0.18444	0.2
0.3 σ_2	0.15	1.17		
D (mm)	300			
E_{v1}	87.48			
E_{v2}	243.88			
Area (sq.m)	0.07065			

$\Delta \sigma$	1.75		
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$$E_v = \frac{P}{\Delta s} \cdot \frac{\Delta \sigma}{\Delta \sigma}$$

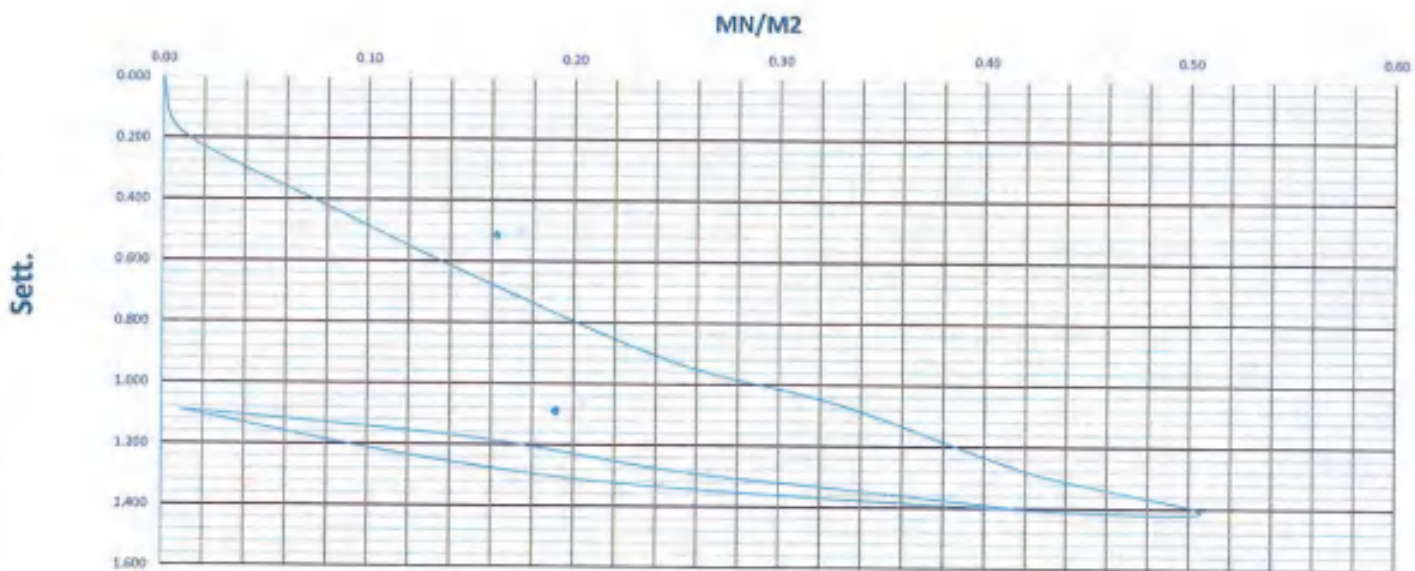
E_v = deformation modulus

$\Delta \sigma$ = load increment

Δs = settlement increment

D = diameter of the plate, generally 0.30 m.

For this calculation $\Delta\sigma$ and Δs are usually taken from the load span between $0.3 \sigma_{max}$ and $0.7 \sigma_{max}$.



Lab. Specialist

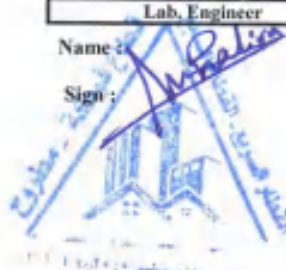
Name :

Sign :

Lab. Engineer

Name :

Sign :



Consultant Engineer

Name :

Sign :

Plate Load Test Results

Company Name

AL MOSTAFA

Location

524+580

To

524+620

Station

524+620

Test Date

9-09-2023

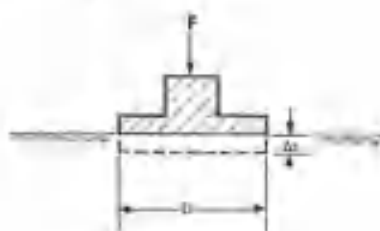
Layer level

P.S.G +0.50

EQUIPMENT AND TEST PROCEDURE :-

The basis of the given equation is Boussinesq's theory of the relationship between the modulus of elasticity and the settlement of a circular rigid plate with the diameter D .

The load is applied to a circular rigid steel bearing plate by a hydraulic jack in several steps. The settlement under each load step is recorded. The following sketch shows the principle of the test.



F = load

Δs = settlement

D = diameter of the plate

The diameter D of the plate is generally 0.30 m. For very coarse grained material also plates with diameter $D = 0.00$ m and $D = 0.762$ m are used.

The load is applied in 6 load increments of equal size. Under each load step the settlement must come to a noticeable end (< 0.02 mm/minute). After the maximum load is reached the unloading procedure can begin. After that, the plate is reloaded in 5 steps. A loaded truck, an excavator or a roller usually serve as counterweight for the hydraulic jack.

Diameter = 300mm

Loading	Load	Load	Stress	Dial 1	Dial 2	Dial 3	Sett. 1	Sett. 2	Sett. 3	Sett. 4
Stage No.	Bar	kN	kN/m ²	mm	mm	mm	mm	mm	mm	mm
0.000	0.0	0.000	0.00	20.00	20.00		0.000	0.000		0.000
1.000	2.1	0.707	0.01	19.90	19.80		0.100	0.200		0.150
2.000	17.1	5.652	0.08	19.65	19.58		0.350	0.420		0.385
3.000	34.2	11.304	0.16	19.38	19.38		0.620	0.620		0.620
4.000	51.3	17.663	0.25	19.07	19.18		0.930	0.820		0.875
5.000	70.5	23.315	0.33	18.94	18.99		1.060	1.010		1.035
6.000	89.8	29.673	0.42	18.65	18.90		1.350	1.100		1.225
7.000	106.8	35.325	0.50	18.47	18.78		1.530	1.220		1.375
8.000	53.4	17.663	0.25	18.52	18.83		1.480	1.170		1.325
9.000	26.7	8.831	0.12	18.59	18.96		1.410	1.040		1.225
9.000	2.1	0.707	0.01	18.75	19.16		1.250	0.840		1.045
10.000	2.1	0.707	0.01	18.75	19.16		1.250	0.840		1.045
11.000	17.1	5.652	0.08	18.70	19.10		1.300	0.900		1.100
12.000	34.2	11.304	0.16	18.65	19.04		1.350	0.960		1.155
13.000	51.3	17.663	0.25	18.55	18.93		1.450	1.070		1.260
14.000	70.5	23.315	0.33	18.48	18.85		1.520	1.150		1.335
15.000	89.8	29.673	0.42	18.43	18.79		1.570	1.210		1.390

		\leq	AS	AS
0.7 σ_0	0.35	1.09375	0.50313	0.2
0.3 σ_0	0.15	0.59063		
0.7 σ_0	0.35	1.34722	0.19222	0.2
0.3 σ_0	0.15	1.15501		
D (mm)	300			
E_v	89.44			
E_v	234.11			
Area (Sq.m)	0.07065			

$F(\Delta E_v)$	2.62		
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$$F_{cr} = 0.75 \cdot R \cdot \pi \cdot D \cdot \Delta s$$

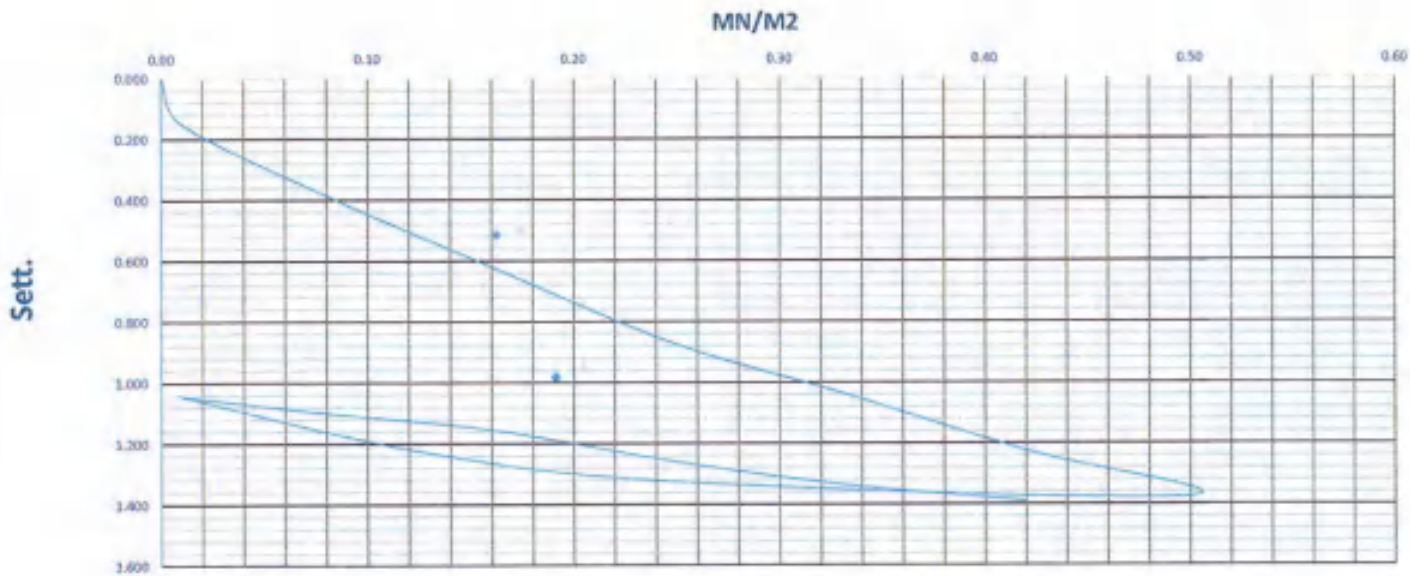
E_v = deformation modulus

Δs = load increment

Δs = settlement increment

D = diameter of the plate, generally 0.30 m

For this calculation $\Delta\sigma$ and Δs are usually taken from the load span between $0.3 \sigma_{max}$ and $0.7 \sigma_{max}$.



Lab. Specialist

Name :

Sign :

Lab. Engineer

Name :

Sign :



Consultant Engineer

Name :

Sign :

Plate Load Test Results

Company Name

AL MOSTAFA

Location

524+580

To

524+660

Station

524+640

Test Date

9-09-2023

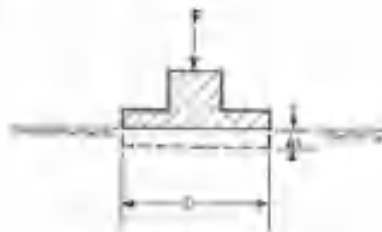
Layer level

P.S.G +0.50

EQUIPMENT AND TEST PROCEDURE :-

The basis of the given equation is Boussinesq's theory of the relationship between the modulus of elasticity and the settlement of a circular rigid plate with the diameter D .

The load is applied to a circular rigid steel bearing plate by a hydraulic jack in several steps. The settlement under each load step is recorded. The following sketch shows the principle of the test.



F = load

s = settlement

D = diameter of the plate

The diameter D of the plate is generally 0.30 m. For very coarse grained material also plates with diameter $D = 0.60$ m and $D = 0.762$ m are used.

The load is applied in 5 load increments of equal size. Under each load step the settlement must come to a noticeable and (< 0.02 mm/minute). After the maximum load is reached the unloading procedure can begin. After that, the plate is reloaded in 5 steps. A loaded truck, an excavator or a roller usually serve as counterweight for the hydraulic jack.

Diameter = 300mm

Loading	Load	Load	Stress	Dist 1	Dist 2	Dist 3	Sett. 1	Sett. 2	Sett. 3	Avg. Sett.
Stage No.	Bar	KN	MPa/M2	mm	mm	mm	mm	mm	mm	mm
0.000	0.0	0.000	0.00	20.00	20.00		0.000	0.000		0.000
1.000	2.1	0.707	0.01	19.85	19.83		0.150	0.170		0.160
2.000	17.1	5.652	0.08	19.63	19.61		0.370	0.390		0.380
3.000	34.2	11.304	0.16	19.35	19.34		0.650	0.660		0.655
4.000	53.3	17.663	0.25	19.17	19.12		0.830	0.880		0.855
5.000	70.5	23.315	0.33	18.98	18.94		1.020	1.060		1.040
6.000	89.8	29.673	0.42	18.68	18.66		1.320	1.140		1.230
7.000	106.8	35.325	0.50	18.46	18.78		1.540	1.220		1.380
8.000	53.4	17.663	0.25	18.52	18.84		1.480	1.160		1.320
9.000	26.7	8.831	0.12	18.61	18.99		1.390	1.010		1.200
9.000	2.1	0.707	0.01	18.72	19.12		1.280	0.880		1.080
10.000	2.1	0.707	0.01	18.72	19.12		1.280	0.880		1.080
11.000	17.1	5.652	0.08	18.68	19.08		1.320	0.920		1.120
12.000	34.2	11.304	0.16	18.62	19.04		1.380	0.960		1.170
13.000	53.3	17.663	0.25	18.55	18.94		1.450	1.060		1.255
14.000	70.5	23.315	0.33	18.49	18.86		1.510	1.140		1.325
15.000	89.8	29.673	0.42	18.45	18.80		1.550	1.200		1.375

		s	Δs	Δs
0.7 σ_1	0.15	1.09875	0.47813	0.3
0.3 σ_1	0.15	0.62063		
0.7 σ_2	0.35	1.33611	0.17611	0.2
0.3 σ_2	0.15	1.10		
D (mm)	300			
E_{s1}	94.22			
E_{s2}	255.53			
Area (sq. m)	0.07065			

E_s / E_{s1}	2.71		
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$$E_s = 0.71 \cdot D \cdot \Delta s / \Delta s$$

E_s = deformation modulus

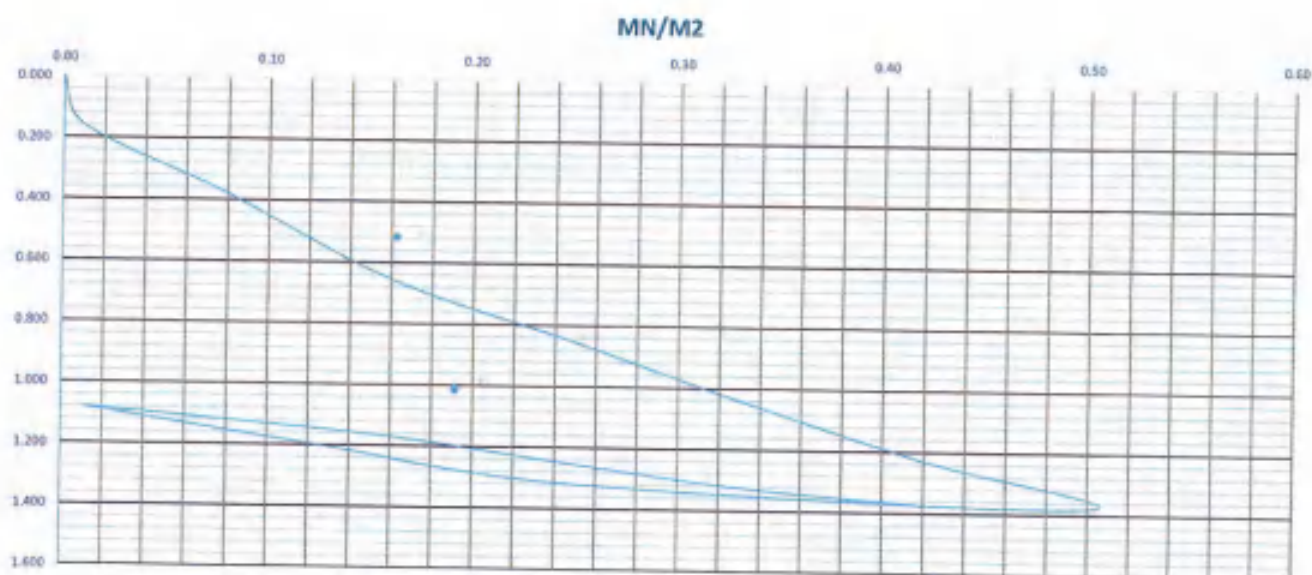
Δs = load increment

Δs = settlement increment

D = diameter of the plate, generally 0.30 m

For this calculation $\Delta\sigma$ and Δs are usually taken from the load span between $0.3 \sigma_{max}$ and $0.7 \sigma_{max}$.

Sett.



Lab. Specialist

Name :

Sign :

Lab. Engineer

Name :

Sign :



Consultant Engineer

Name :

Sign :

Plate Load Test Results

Company Name

AL MOSTAFA

Location

524+660

To

524+740

Station

524+665

Test Date

8-09-2023

Layer level

P.S.G +0.50

EQUIPMENT AND TEST PROCEDURE :-

The basis of the given equation is Boussinesq's theory of the relationship between the modulus of elasticity and the settlement of a circular rigid plate with the diameter D .

The load is applied to a circular rigid steel bearing plate by a hydraulic jack in several steps. The settlement under each load step is recorded. The following sketch shows the principle of the test.



P = load

s = settlement

D = diameter of the plate

The diameter D of the plate is generally 0.30 m. For very coarse grained material also plates with diameter $D = 0.60$ m and $D = 0.762$ m are used.

The load is applied in 6 load increments of equal size. Under each load step the settlement must come to a noticeable end (< 0.02 mm/minute). After the maximum load is reached the unloading procedure can begin. After that, the plate is reloaded in 5 steps. A loaded truck, an excavator or a roller usually serve as counterweight for the hydraulic jack.

Diameter = 300mm

Loading	Level	Load	Stress	Dial 1	Dial 2	Dial 1	Sett. 1	Sett. 2	Sett. 3	Avg. Sett.
Stage No.	mm	kN	MPa/ksi	mm	mm	mm	mm	mm	mm	mm
0.000	0.0	0.000	0.00	20.00	20.00		0.000	0.000		0.000
1.000	2.1	0.707	0.01	19.86	19.85		0.140	0.150		0.145
2.000	17.1	5.652	0.08	19.71	19.60		0.290	0.400		0.345
3.000	34.2	11.304	0.16	19.47	19.30		0.530	0.700		0.615
4.000	53.3	17.663	0.25	19.30	19.10		0.700	0.900		0.800
5.000	70.5	23.315	0.33	19.10	18.93		0.900	1.070		0.985
6.000	89.8	29.673	0.42	18.95	18.80		1.050	1.200		1.125
7.000	106.8	35.325	0.50	18.81	18.73		1.190	1.270		1.230
8.000	53.4	17.663	0.25	18.83	18.75		1.170	1.250		1.210
9.000	26.7	8.831	0.12	18.90	18.81		1.100	1.190		1.145
9.000	2.1	0.707	0.01	18.98	18.93		1.020	1.070		1.045
10.000	2.1	0.707	0.01	18.98	18.93		1.020	1.070		1.045
11.000	17.1	5.652	0.08	18.97	18.92		1.030	1.080		1.055
12.000	34.2	11.304	0.16	18.93	18.86		1.070	1.140		1.105
13.000	53.3	17.663	0.25	18.90	18.80		1.100	1.200		1.150
14.000	70.5	23.315	0.33	18.88	18.76		1.120	1.240		1.180
15.000	89.8	29.673	0.42	18.84	18.72		1.160	1.280		1.220

		s	AS	AS
0.7 σ_1	0.35	1.03313	0.45188	0.2
0.3 σ_2	0.15	0.58125		
0.7 σ_2	0.35	1.18889	0.12389	0.2
0.3 σ_2	0.15	1.005		
D (mm)	300			
E_{v1}	99.50			
E_{v2}	763.23			
Area (Sq.m)	0.07065			

E_{v2}/E_{v1}	7.65		
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$$E_v = 0.75 \cdot D \cdot \Delta s / \Delta s$$

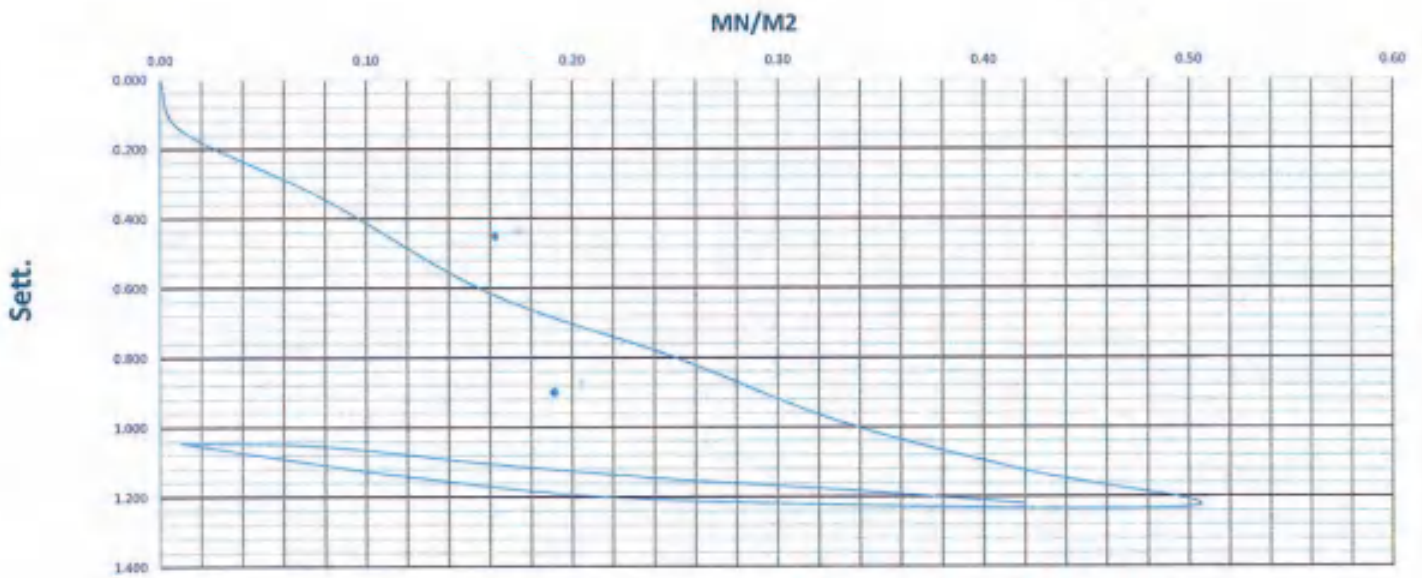
E_v = deformation modulus

Δs = load increment

Δs = settlement increment

D = diameter of the plate, generally 0.30 m

For this calculation $\Delta\sigma$ and Δs are usually taken from the load span between $0.3 \sigma_{max}$ and $0.7 \sigma_{max}$.



Lab. Specialist

Name :

Sign :

Lab. Engineer

Name :

Sign :



Consultant Engineer

Name :

Sign :

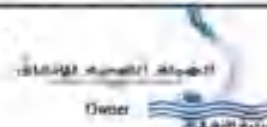


Plate Load Test Results

Company Name

AL MOSTAFA

Location

524+660

To

524+740

Station

524+690

Test Date

8-09-2023

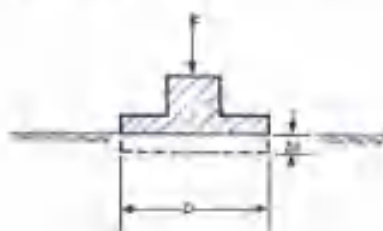
Layer level

P.S.G +0.50

EQUIPMENT AND TEST PROCEDURE :-

The basis of the given equation is Boussinesq's theory of the relationship between the modulus of elasticity and the settlement of a circular rigid plate with the diameter D .

The load is applied to a circular rigid steel bearing plate by a hydraulic jack in several steps. The settlement under each load step is recorded. The following sketch shows the principle of the test.



$L = \text{load}$

$\Delta s = \text{settlement}$

$D = \text{diameter of the plate}$

The diameter D of the plate is generally 0.30 m. For very coarse grained material also plates with diameter $D = 0.60$ m and $D = 0.762$ m are used.

The load is applied in 6 load increments of equal size. Under each load step the settlement must come to a noticeable end (< 0.02 mm/minute). After the maximum load is reached the unloading procedure can begin. After that, the plate is reloaded in 5 steps. A loaded truck, an excavator or a roller usually serve as counterweight for the hydraulic jack.

Diameter = 300mm

Loading	Load	Load	Stress	Def 1	Def 2	Def 3	Sett. 1	Sett. 2	Sett. 3	Avg. Sett.
Stage No.	Bar	KN	MPa	mm	mm	mm	mm	mm	mm	mm
0.000	0.0	0.000	0.00	20.00	20.00		0.000	0.000		0.000
1.000	2.1	0.707	0.01	19.89	19.92		0.110	0.080		0.095
2.000	17.1	5.652	0.08	19.73	19.63		0.270	0.370		0.320
3.000	34.2	11.304	0.16	19.45	19.36		0.550	0.640		0.595
4.000	53.3	17.663	0.25	19.32	19.14		0.680	0.860		0.770
5.000	70.5	23.315	0.33	19.13	18.95		0.870	1.050		0.960
6.000	89.8	29.673	0.42	18.97	18.84		1.030	1.160		1.095
7.000	106.8	35.325	0.50	18.89	18.70		1.200	1.300		1.250
8.000	53.4	17.663	0.25	18.85	18.77		1.150	1.230		1.190
9.000	26.7	8.831	0.12	18.92	18.83		1.080	1.170		1.125
9.000	2.1	0.707	0.01	18.97	18.94		1.030	1.060		1.045
10.000	2.1	0.707	0.01	18.97	18.94		1.030	1.060		1.045
11.000	17.1	5.652	0.08	18.95	18.93		1.050	1.070		1.060
12.000	34.2	11.304	0.16	18.92	18.87		1.080	1.130		1.105
13.000	53.3	17.663	0.25	18.90	18.82		1.100	1.180		1.140
14.000	70.5	23.315	0.33	18.87	18.75		1.130	1.250		1.190
15.000	89.8	29.673	0.42	18.81	18.70		1.190	1.300		1.245

		ν	Δs	Δs
0.7 σ_1	0.35	0.95938	0.39875	0.2
0.3 σ_1	0.15	0.55863		
0.7 σ_2	0.35	1.20222	0.12722	0.2
0.3 σ_2	0.15	1.075		
D (mm)	300			
E_{v1}	112.85			
E_{v2}	453.72			
Area (sq.m)	0.07065			

E_{v2}/E_{v1}	4.13		
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$$E_v = 0.75 \times D \times \Delta s / \Delta s$$

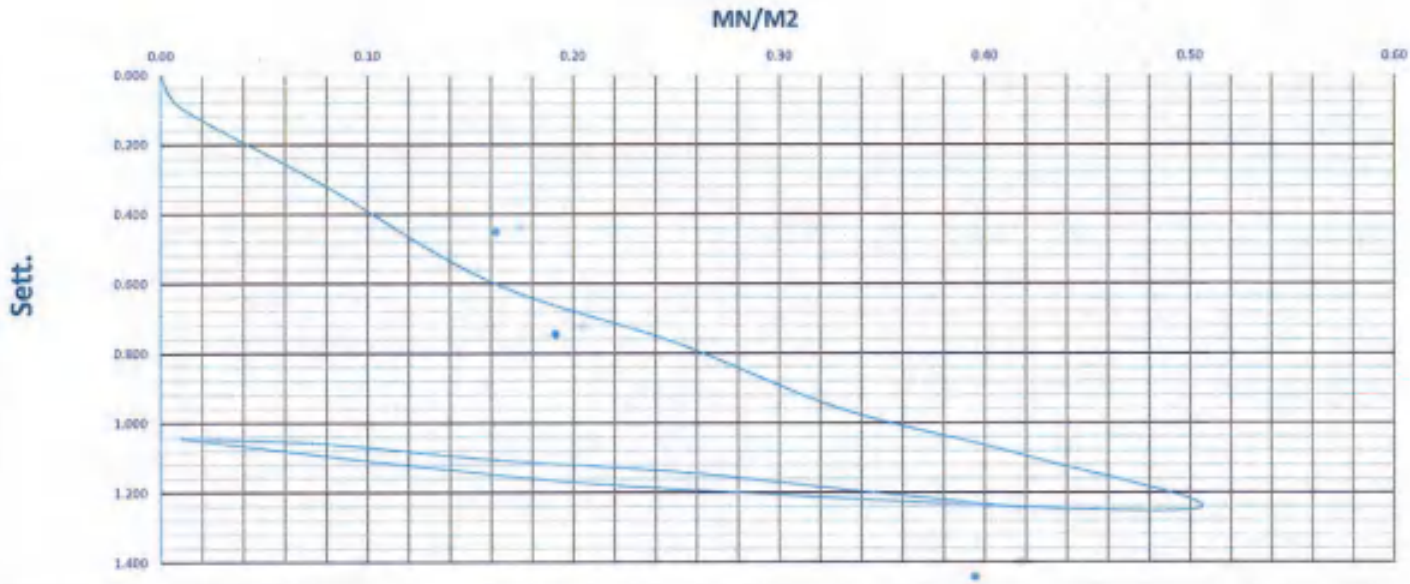
E_v = deformation modulus

Δs = load increment

Δs = settlement increment

D = diameter of the plate, generally 0.30 m

For this calculation $\Delta\sigma$ and Δs are usually taken from the load span between $0.3 \sigma_{max}$ and $0.7 \sigma_{max}$.



Lab. Specialist

Name :

Sign :

Lab. Engineer

Name :

Sign :



Consultant Engineer

Name :

Sign :



Plate Load Test Results

Company Name

AL MOSTAFA

Location

524+660

To

524+740

Status

524+720

Test Date

8-09-2023

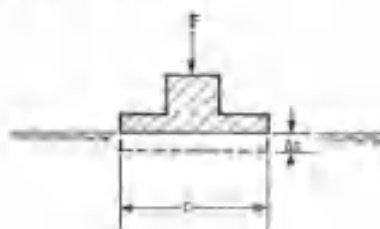
Layer level

P.S.G +0.50

EQUIPMENT AND TEST PROCEDURE :-

The basis of the given equation is Boussinesq's theory of the relationship between the modulus of elasticity and the settlement of a circular rigid plate with the diameter D .

The load is applied in a circular rigid steel bearing plate by a hydraulic jack in several steps. The settlement under each load step is recorded. The following sketch shows the principle of the test.



$F = \text{load}$

$\Delta s = \text{settlement}$

$D = \text{diameter of the plate}$

The diameter D of the plate is generally 0.30 m. For very coarse grained material also plates with diameter $D = 0.60$ m and $D = 0.762$ m are used.

The load is applied in 6 load increments of equal size. Under each load step the settlement must come to a noticeable and (< 0.02 mm/minute). After the maximum load is reached the unloading procedure can begin. After that, the plate is reloaded in 5 steps. A loaded truck, an excavator or a roller usually serve as counterweight for the hydraulic jack.

Diameter = 300mm

Loading	Load	Load	Stress	Dial 1	Dial 2	Dial 3	Sett. 1	Sett. 2	Sett. 3	Avg. Sett.
Stage No.	Bar	kN	MPa	mm	mm	mm	mm	mm	mm	mm
0.000	0.0	0.000	0.00	20.00	20.00		0.000	0.000		0.000
1.000	2.1	0.707	0.01	19.92	19.90		0.080	0.100		0.090
2.000	17.1	5.652	0.08	19.74	19.59		0.260	0.410		0.335
3.000	34.2	11.304	0.16	19.50	19.28		0.500	0.720		0.610
4.000	53.3	17.663	0.25	19.25	19.16		0.750	0.840		0.795
5.000	70.5	23.315	0.33	19.17	18.91		0.830	1.090		0.960
6.000	89.8	29.673	0.42	18.94	18.81		1.060	1.190		1.125
7.000	106.8	35.325	0.50	18.70	18.68		1.300	1.320		1.310
8.000	53.4	17.663	0.25	18.80	18.75		1.200	1.250		1.225
9.000	26.7	8.831	0.12	18.90	18.80		1.100	1.200		1.150
9.000	2.1	0.707	0.01	19.05	18.94		0.950	1.060		1.005
10.000	2.1	0.707	0.01	19.05	18.94		0.950	1.060		1.005
11.000	17.1	5.652	0.08	18.98	18.90		1.020	1.100		1.060
12.000	34.2	11.304	0.16	18.90	18.85		1.100	1.150		1.125
13.000	53.3	17.663	0.25	18.81	18.82		1.190	1.180		1.185
14.000	70.5	23.315	0.33	18.82	18.73		1.180	1.270		1.225
15.000	89.8	29.673	0.42	18.74	18.70		1.260	1.300		1.280

	σ	Δs	Δs
0.7 σ_1	0.35	0.96313	0.3875
0.3 σ_1	0.15	0.57563	
0.7 σ_2	0.35	1.23722	0.12222
0.3 σ_2	0.15	1.11501	
D (mm)	300		
E_{v1}	116.13		
E_{v2}	360.20		
Area (Sq.m)	0.07065		

E_{v2}/E_{v1}	3.11		
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$$E_v = 0.73 \times D \times \Delta s / \Delta s$$

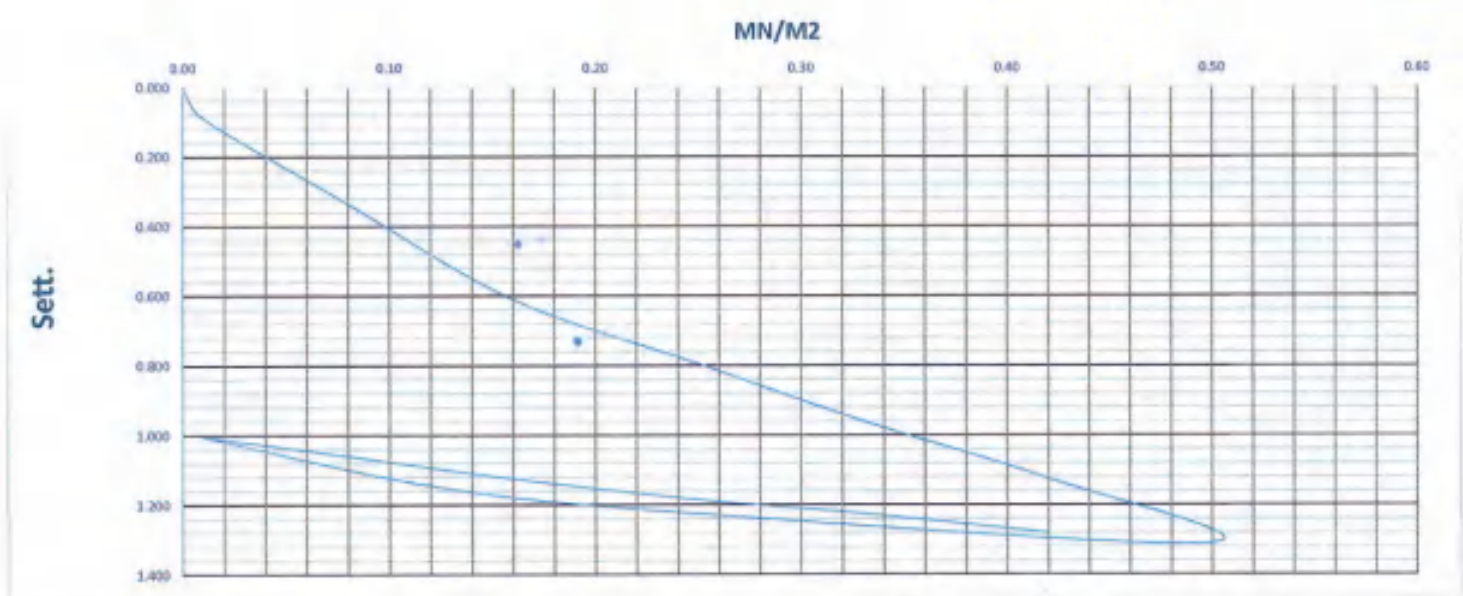
E_v = deformation modulus

Δs = load increment


Δs = settlement increment

D = diameter of the plate, generally 0.30 m

For this calculation $\Delta\sigma$ and Δx are usually taken from the load span between $0.3 \sigma_{max}$ and $0.7 \sigma_{max}$.



Lab. Specialist
Name :
Sign :

Lab. Engineer
Name :
Sign :


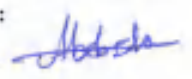
Consultant Engineer
Name :
Sign : 

Plate Load Test Results

Company Name

AL MOSTAFA

Location

524+740

To

524+820

Station

524+760

Test Date

7-09-2023

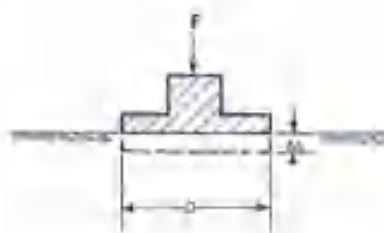
Layer level

P.S.G +0.50

EQUIPMENT AND TEST PROCEDURE :-

The basis of the given equation is Boussinesq's theory of the relationship between the modulus of elasticity and the settlement of a circular rigid plate with the diameter D .

The load is applied to a circular rigid steel bearing plate by a hydraulic jack in several steps. The settlement under each load step is recorded. The following sketch shows the principle of the test.



F = load

s = settlement

D = diameter of the plate (mm)

The diameter D of the plate is generally 0.30 m. For very coarse grained material also plates with diameter $D = 0.60$ m and $D = 0.762$ m are used.

The load is applied in 5 load increments of equal size. Under each load step the settlement must come to a noticeable end (< 0.02 mm/minute). After the maximum load is reached the unloading procedure can begin. After that, the plate is reloaded in 5 steps. A loaded truck, an excavator or a roller usually serve as counterweight for the hydraulic jack.

Diameter = 300mm

Loading	Load	Load	Series	Dial 1	Dial 2	Dial 3	Sett. 1	Sett. 2	Sett. 3	Avg. Sett.
Stage No.	Bar	kg	30/002	mm	mm	mm	mm	mm	mm	mm
0.000	0.0	0.000	0.00	20.00	20.00		0.000	0.000		0.000
1.000	2.1	0.707	0.01	19.92	19.95		0.080	0.050		0.065
2.000	17.1	5.652	0.08	19.80	19.87		0.200	0.130		0.165
3.000	34.2	11.304	0.16	19.60	19.80		0.400	0.200		0.300
4.000	53.3	17.663	0.25	19.32	19.70		0.680	0.300		0.490
5.000	70.5	23.315	0.33	19.20	19.65		0.800	0.350		0.575
6.000	89.8	29.673	0.42	19.05	19.60		0.950	0.400		0.675
7.000	106.8	35.325	0.50	18.91	19.59		1.090	0.410		0.750
8.000	53.4	17.663	0.28	18.96	19.65		1.040	0.350		0.695
9.000	26.7	8.831	0.12	19.07	19.70		0.930	0.300		0.615
9.000	2.1	0.707	0.01	19.28	19.79		0.720	0.210		0.465
10.000	2.1	0.707	0.01	19.28	19.79		0.720	0.210		0.465
11.000	17.1	5.652	0.08	19.26	19.78		0.740	0.220		0.480
12.000	34.2	11.304	0.16	19.15	19.75		0.850	0.250		0.550
13.000	53.3	17.663	0.25	19.10	19.70		0.900	0.300		0.600
14.000	70.5	23.315	0.33	19.04	19.65		0.960	0.350		0.655
15.000	89.8	29.673	0.42	18.96	19.58		1.040	0.420		0.730

D (mm)	s	AS	AS	AS
0.7 σ_1	0.35	0.60937	0.32625	0.2
0.3 σ_1	0.15	0.28512		
0.7 σ_2	0.35	0.67167	0.17967	0.2
0.3 σ_2	0.15	0.495		
D (mm)	300			
E_{v1}	137.03			
E_{v2}	154.72			
Area (Sq.m)	0.07065			

E_{v2}/E_{v1}	1.05		
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$$E_v = \frac{D \cdot \Delta \sigma}{\Delta s}$$

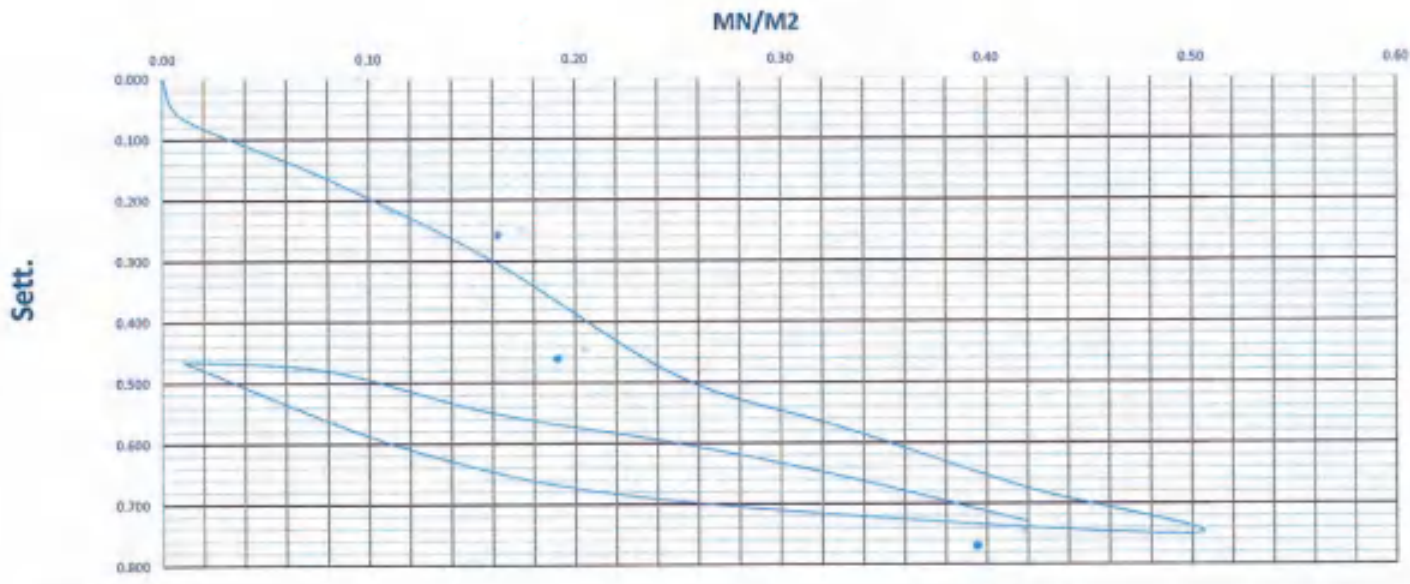
E_v = deformation modulus

$\Delta \sigma$ = load increment

Δs = settlement increment

D = diameter of the plate, generally 0.30 m

For this calculation $\Delta\sigma$ and Δs are usually taken from the load span between $0.3 \sigma_{max}$ and $0.7 \sigma_{max}$.



Lab. Specialist

Name :

Sign :

Lab. Engineer

Name :

Sign :

Consultant Engineer

Name :

Sign :





Plate Load Test Results

Company Name AL MOSTAFA

Location

524+740

To

524+820

Station

524+780

Test Date

7-09-2023

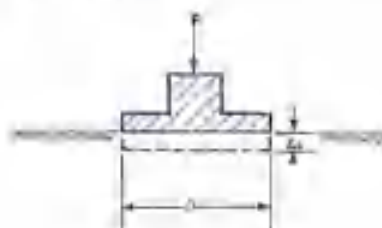
Layer level

P.S.G +0.50

EQUIPMENT AND TEST PROCEDURE :-

The basis of the given equation is Boussinesq's theory of the relationship between the modulus of elasticity and the settlement of a circular rigid plate with the diameter D .

The load is applied to a circular rigid steel bearing plate by a hydraulic jack in several steps. The settlement under each load step is recorded. The following sketch shows the principle of the test.



P = load

s = settlement

D = diameter of test plate

The diameter D of the plate is generally 0.30 m. For very coarse grained material also plates with diameter $D = 0.60$ m and $D = 0.762$ m are used.

The load is applied in 5 load increments of equal size. Under each load step the settlement must come to a noticeable end (< 0.02 mm/minute). After the maximum load is reached the unloading procedure can begin. After that, the plate is reloaded in 5 steps. A loaded truck, an excavator or a roller usually serve as counterweight for the hydraulic jack.

Diameter = 300mm

Loading	Load	Load	Stress	Dial 1	Dial 2	Dial 3	Sett. 1	Sett. 2	Sett. 3	Avg. Sett.
Stage No.	Bar	kN	MS/M2	mm	mm	mm	mm	mm	mm	mm
0.000	0.0	0.000	0.00	20.00	20.00		0.000	0.000		0.000
1.000	2.1	0.707	0.01	19.90	19.92		0.100	0.080		0.090
2.000	17.1	5.652	0.08	19.70	19.85		0.300	0.150		0.225
3.000	34.2	11.304	0.16	19.52	19.78		0.480	0.220		0.350
4.000	53.3	17.663	0.25	19.32	19.68		0.680	0.320		0.500
5.000	70.5	23.315	0.33	19.16	19.64		0.820	0.360		0.590
6.000	89.8	29.673	0.42	19.02	19.60		0.980	0.400		0.690
7.000	106.8	35.325	0.50	18.88	19.55		1.120	0.450		0.785
8.000	53.4	17.663	0.25	18.97	19.65		1.030	0.350		0.690
9.000	26.7	8.831	0.12	19.10	19.72		0.900	0.280		0.590
9.000	2.1	0.707	0.01	19.30	19.80		0.700	0.200		0.450
10.000	2.1	0.707	0.01	19.30	19.80		0.700	0.200		0.450
11.000	17.1	5.652	0.08	19.25	19.77		0.750	0.230		0.490
12.000	34.2	11.304	0.16	19.16	19.74		0.840	0.260		0.550
13.000	53.3	17.663	0.25	19.06	19.70		0.940	0.300		0.620
14.000	70.5	23.315	0.33	19.00	19.66		1.000	0.340		0.670
15.000	89.8	29.673	0.42	18.92	19.59		1.080	0.410		0.745

		s	AS	Δs
0.7 σ_1	0.35	0.60687	0.2725	0.2
0.3 σ_1	0.15	0.33438		
0.7 σ_2	0.35	0.88667	0.15666	0.2
0.3 σ_2	0.15	0.53		
D (mm)	300			
E_{v1}	105.14			
E_{v2}	287.24			
Area (sq.m)	0.07065			

E_{v2}/E_{v1}	1.74		
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$$E_v = 0.75 \cdot D \cdot \Delta s / \Delta s$$

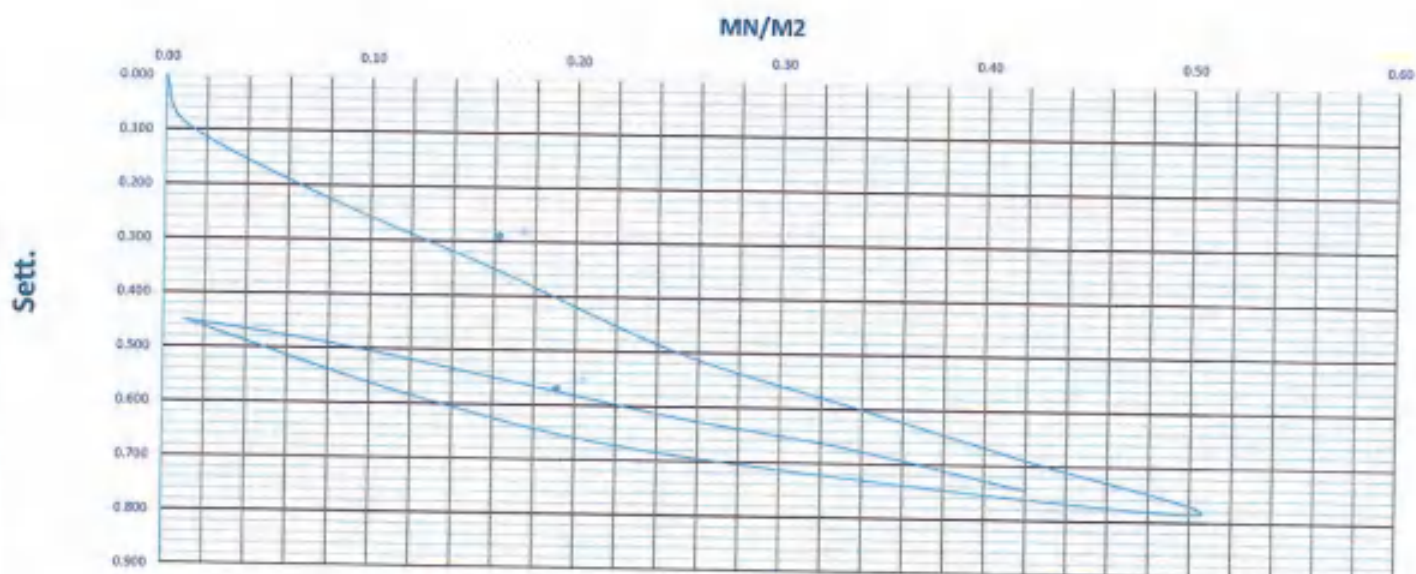
E_v = deformation modulus

Δs = load increment

Δs = settlement increment

D = diameter of the plate, generally 0.30 m

For this calculation $\Delta\sigma$ and Δs are usually taken from the load span between $0.3 \sigma_{max}$ and $0.7 \sigma_{max}$.



Lab. Specialist

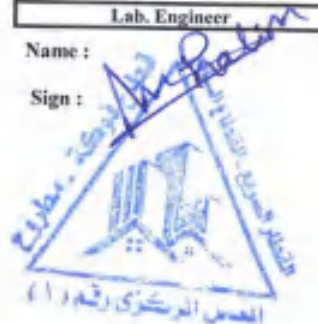
Name :

Sign :

Lab. Engineer

Name :

Sign :



Consultant Engineer

Name :

Sign :

Abdelhadi

Plate Load Test Results

Company Name

AL MOSTAFA

Location

524+740

To

524+820

Station

524+800

Test Date

7-09-2023

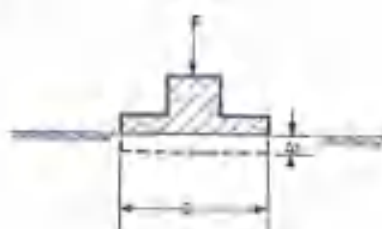
Layer level

P.S.G +0.50

EQUIPMENT AND TEST PROCEDURE

The basis of the given equation is Boussinesq's theory of the relationship between the modulus of elasticity and the settlement of a circular rigid plate with the diameter D .

The load is applied to a circular rigid steel bearing plate by a hydraulic jack in several steps. The settlement under each load step is recorded. The following sketch shows the principle of the test.



P = load

Δs = settlement

D = diameter of the plate

The diameter D of the plate is generally 0.30 m. For very coarse grained material also plates with diameter $D = 0.60$ m and $D = 0.762$ m are used.

The load is applied in 8 load increments of equal size. Under each load step the settlement must come to a noticeable end (< 0.02 mm/minute). After the maximum load is reached the unloading procedure can begin. After that, the plate is reloading in 5 steps. A loaded truck, an excavator or a roller usually serve as counterweight for the hydraulic jack.

Diameter = 300mm

Loading	Load	Load	Stress	Dial 1	Dial 2	Dial 3	Sett. 1	Sett. 2	Sett. 3	Av. Sett.
Stage No.	Bar	KN	KN/M ²	mm	mm	mm	mm	mm	mm	mm
0.000	0.0	0.000	0.00	20.00	20.00		0.000	0.000		0.000
1.000	2.1	0.707	0.01	19.97	19.98		0.030	0.020		0.025
2.000	17.1	5.652	0.08	19.03	19.88		0.170	0.120		0.145
3.000	34.2	11.304	0.16	19.50	19.75		0.500	0.250		0.375
4.000	51.3	17.663	0.25	19.30	19.67		0.700	0.330		0.515
5.000	70.5	23.315	0.33	19.15	19.64		0.850	0.360		0.605
6.000	89.8	29.673	0.42	19.00	19.58		1.000	0.420		0.710
7.000	106.8	35.325	0.50	18.85	19.52		1.150	0.480		0.815
8.000	53.4	17.603	0.25	18.90	19.60		1.100	0.400		0.750
9.000	26.7	8.831	0.12	19.00	19.72		1.000	0.280		0.640
9.000	2.1	0.707	0.01	19.20	19.80		0.800	0.200		0.500
10.000	2.1	0.707	0.01	19.20	19.80		0.800	0.200		0.500
11.000	17.1	5.652	0.08	19.18	19.70		0.820	0.300		0.560
12.000	34.2	11.304	0.16	19.10	19.65		0.900	0.350		0.625
13.000	51.3	17.663	0.25	19.06	19.57		0.940	0.430		0.685
14.000	70.5	23.315	0.33	19.00	19.52		1.000	0.480		0.740
15.000	89.8	29.673	0.42	18.92	19.45		1.080	0.550		0.815

		s	ΔS	Δσ
0,7 σ ₁	0,35	0,61813	0,27188	0,2
0,5 σ ₁	0,15	0,34625		
0,7 σ ₂	0,35	0,75607	0,13666	0,2
0,5 σ ₂	0,15	0,82001		
D (mm)	300			
E _{s1}	165,52			
E _{s2}	329,28			
Area (kg/m ²)	0,07067			

E_s (kN)	1.90		
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$E_s = 0.75 \cdot D \cdot \Delta s / \Delta s$	
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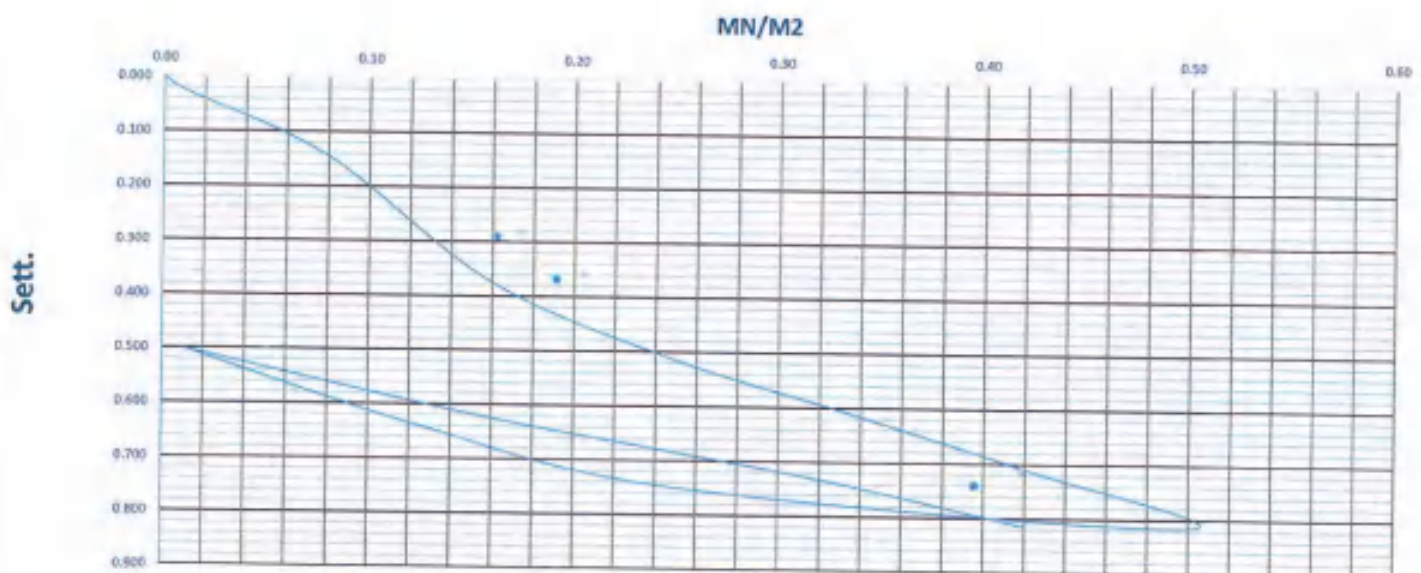
E_s = deformation modulus

Δs = load increment

Δs = settlement increment

D = diameter of the plate, generally 0.30 m

For this calculation $\Delta\sigma$ and Δx are usually taken from the load span between $0.3 \sigma_{max}$ and $0.7 \sigma_{max}$.



Lab. Specialist

Name :

Sign :

Lab. Engineer

Name :

Sign :



Consultant Engineer

Name :

Sign :

Plate Load Test Results

Company Name

AL MOSTAFA

Location

524+740

To

524+820

Station

524+815

Test Date

7-09-2023

Layer level

P.S.G +0.50

EQUIPMENT AND TEST PROCEDURE :-

The basis of the given equation is Boussinesq's theory of the relationship between the modulus of elasticity and the settlement of a circular rigid plate with the diameter D .

The load is applied to a circular rigid steel bearing plate by a hydraulic jack in several steps. The settlement under each load step is recorded. The following sketch shows the principle of the test.



F = load

Δs = settlement

D = diameter of the plate

The diameter D of the plate is generally 0.30 m. For very coarse grained material also plates with diameter $D = 0.60$ m and $D = 0.762$ m are used.

The load is applied in 6 load increments of equal size. Under each load step the settlement must come to a noticeable end (< 0.02 mm/minute). After the maximum load is reached the unloading procedure can begin. After that, the plate is reloaded in 5 steps. A loaded truck, an excavator or a roller usually serve as counterweight for the hydraulic jack.

Diameter = 300mm

Loading	Load	Load	Strain	Dial 1	Dial 2	Dial 3	Sett. 1	Sett. 2	Sett. 3	Avg. Sett.
Stage No.	Bar	KN	MM/M2	mm	mm	mm	mm	mm	mm	mm
0.000	0.0	0.000	0.00	20.00	20.00		0.000	0.000		0.000
1.000	2.1	0.707	0.01	19.97	19.96		0.030	0.040		0.035
2.000	17.1	5.652	0.08	19.80	19.80		0.200	0.200		0.200
3.000	34.2	11.304	0.16	19.50	19.55		0.500	0.450		0.475
4.000	53.3	17.663	0.25	19.25	19.31		0.750	0.690		0.720
5.000	70.5	23.315	0.33	19.05	19.12		0.950	0.880		0.915
6.000	89.8	29.673	0.42	18.84	18.93		1.160	1.070		1.115
7.000	106.8	35.325	0.50	18.62	18.60		1.380	1.400		1.390
8.000	53.4	17.663	0.25	18.72	18.68		1.280	1.320		1.300
9.000	26.7	8.831	0.12	18.80	18.90		1.200	1.100		1.150
9.000	2.1	0.707	0.01	18.93	19.07		1.070	0.930		1.000
10.000	2.1	0.707	0.01	18.93	19.07		1.070	0.930		1.000
11.000	17.1	5.652	0.08	18.92	19.04		1.080	0.960		1.020
12.000	34.2	11.304	0.16	18.88	18.90		1.120	1.100		1.110
13.000	53.3	17.663	0.25	18.82	18.82		1.180	1.180		1.180
14.000	70.5	23.315	0.33	18.75	18.74		1.250	1.260		1.255
15.000	89.8	29.673	0.42	18.70	18.66		1.300	1.340		1.320

		ϕ	AS	AS
0.7 σ_1	0.35	0.87438	0.43375	0.2
0.3 σ_1	0.15	0.44063		
0.7 σ_1	0.35	1.26944	0.22944	0.2
0.3 σ_1	0.15	1.04		
D (mm)	300			
E_{v1}	103.25			
E_{v2}	196.13			
Area (sq m)	0.07065			

$K(2E_v)$	1.09		
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$E_s = 0.75 \cdot R \cdot \Delta s / \Delta s$	
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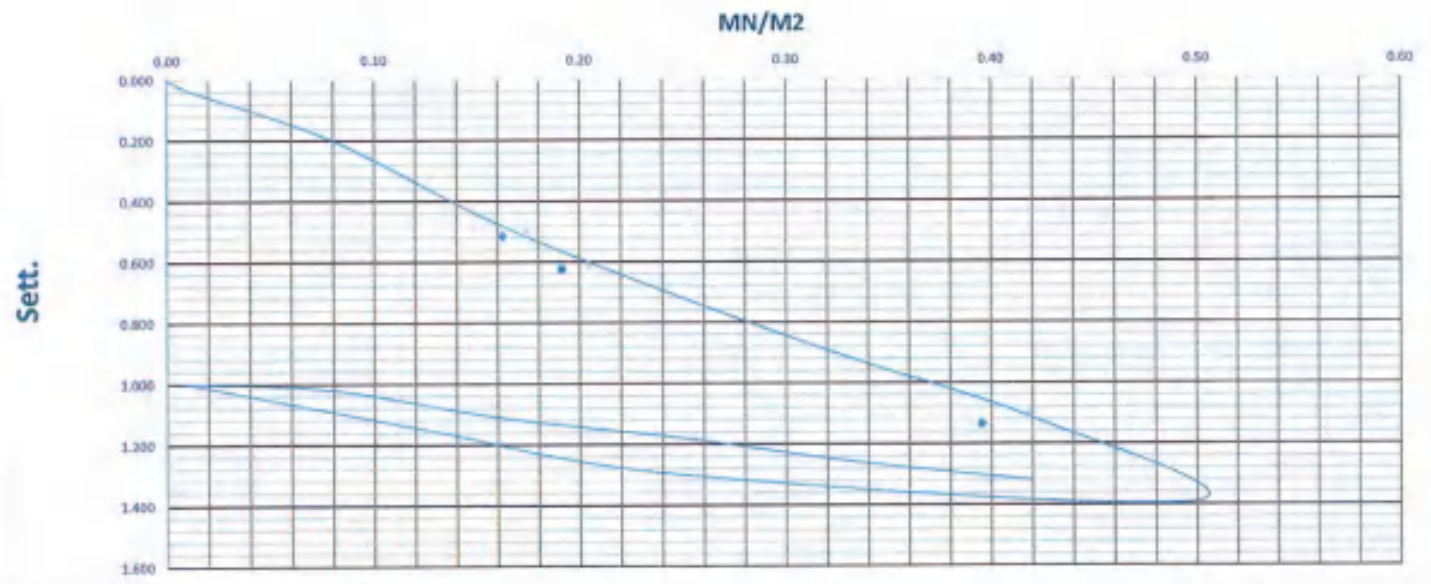
E_s = deformation modulus

Δs = load increment

D_s = settlement increment

D = diameter of the plate, generally 0.30 m

For this calculation $\Delta\sigma$ and Δs are usually taken from the load span between $0.3 \sigma_{max}$ and $0.7 \sigma_{max}$.



Lab. Specialist

Name :

Sign :

Lab. Engineer

Name :

Sign :



Consultant Engineer

Name :

Sign :

Abdullah

☐ **Wavelength:**

www.elsevier.com/locate/jmb

Station	328-001
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EQUIPMENT AND TEST PROCEDURE:

The diameter D of the plate is generally 0.30 m. For very coarse grained material also plates with diameter $D = 0.60$ m and $D = 0.762$ m are used.

The load is applied in 5 load increments of equal size. Under each load step the settlement must come to a noticeable end (i.e. 0.02 mm/minute). After the maximum load is reached the unloading procedure can begin. After that, the plate is reloaded in 5 steps. A loaded truck, an excavator or a roller usually serve as counterweight for the hydraulic jack.

Diameter = 300 mm

$$B_{\text{eff}} = 0.75 \cdot B \cdot \left(\frac{\text{len}}{f} \right) \cdot \Delta q$$

Figure 1 = Discrimination measures

Figure 1 = Discrimination measures

Q. = brief interview

D4 ← Selbstverständliches

D — diameter of the plate, generally 0.30 m;

γ is the logarithm of the semi- γ . It is usually close to the real space lattice $\Gamma = \frac{1}{2\pi} \log \gamma$ and it is γ_{max} .

W4/M3

Soft

Consultant Engineer

Name: James R. R. R. R.

Sign : *[Signature]*



المملكة العربية السعودية
الجمهورية العربية السورية

Client/Customer	Contracting Consultant	CENTRAL LAB	Collaboration	Order
Plate Load Test Results				
Company Name	AL.MOSTAFA			
Location	524 + 760	To	524 + 900	Station
Test Date	1-10-2023			
Layer level	SUB BALLAST +0.90			

EQUIPMENT AND TEST PROCEDURE :-

The diameter D of the plate is generally 0.30 m. For very coarse grained material also plates with diameter $D = 0.60$ m and $D = 0.762$ m are used.

The load is applied in 8 load increments of equal size. Under each load step the settlement must come to a noticeable end (< 0.02 mm/min). After the maximum load is reached the unloading procedure can begin. After that, the plate is reloaded in 8 steps. A loaded truck, an excavator or a roller usually serve as counterweight for the hydraulic jack.

Diameter = 300mm

Load	Load	Force	Load	Load	Load	Sett	Sett	Sett	Sett
mm	mm	kN	mm	mm	mm	mm	mm	mm	mm
0.000	0.0	0.000	0.00	20.00	20.00	0.000	0.000		0.000
1.000	2.1	0.707	0.01	18.96	18.97	0.040	0.030		0.035
2.000	17.1	5.852	0.08	19.78	19.88	0.220	0.120		0.170
3.000	34.2	11.704	0.16	19.39	19.78	0.410	0.220		0.315
4.000	51.3	17.556	0.25	19.40	19.62	0.600	0.380		0.490
5.000	78.3	23.408	0.35	19.21	19.50	0.770	0.540		0.635
6.000	105.4	29.260	0.42	19.12	19.44	0.860	0.560		0.720
7.000	132.4	35.112	0.50	19.02	19.37	0.980	0.630		0.805
8.000	159.5	40.964	0.25	19.06	19.41	0.840	0.590		0.765
9.000	186.6	46.816	0.12	19.14	19.48	0.860	0.520		0.690
10.000	213.7	52.668	0.01	19.34	19.61	0.660	0.390		0.525
11.000	240.8	58.520	0.01	19.34	19.61	0.660	0.390		0.525
12.000	267.9	64.372	0.08	19.32	19.55	0.880	0.420		0.585
13.000	295.0	70.224	0.14	19.21	19.52	0.750	0.480		0.615
14.000	322.1	76.076	0.25	19.14	19.47	0.860	0.530		0.695
15.000	349.2	81.928	0.35	19.06	19.41	0.940	0.580		0.785
16.000	376.3	87.780	0.42	19.02	19.38	0.880	0.620		0.800

	σ	ΔS	$\Delta \sigma$
$0.7 \sigma_1$	0.35	0.64582	0.34415
$0.3 \sigma_1$	0.15	0.28688	
$R_{T0.1}$	0.35	0.77279	0.16777
$R_{T0.2}$	0.15	0.805	
D (mm)	300		
E_s	129.03		
E_v	258.25		
Area (sqm)	0.07065		

E_s/E_v	0.5		
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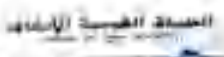
$$E_s = 0.75 \cdot D \cdot \Delta \sigma / \Delta S$$

- E_s = deformation modulus
- D = load increment
- $\Delta \sigma$ = settlement increment
- D = diameter of the plate, generally 0.30 m

Fig. 10.10 Plate Load Test Results. The results are from the test plate. The results are from the test plate.

10/10/23

Lab Specialist	Lab Engineer	Consultant Engineer
Name :	Name : AHMED HALEEM	Name : Fawaz Rashed
Sign :	Sign :	Sign :

Client Details	Contract Details	CENTRAL LAB	District	Region
Plate Load Test Results				
Company Name	AL MOSTAFA			
Location	524 + 760	To	524 + 900	
Test Date	28-09-2023			
Layer level	SUB BALLAST +0.90			

Station: 524+845

EQUIPMENT AND TEST PROCEDURE :-

The diameter D of the plate is generally 0.30 m, for very coarse grained material also plates with diameter $D = 0.50$ m and $D = 0.70$ m are used.

The load is applied in 8 load increments of equal size. Under each load step the settlement must come to a noticeable end (≈ 0.02 mm/minute). After the maximum load is reached the unloading procedure can begin. After that, the plate is reloaded in 5 steps. A loaded truck, an excavator or a roller usually serve as counterweight for the hydraulic jack.

Diameter = 300mm

Loading	Load	Load	Sett.	Dist. 1	Dist. 2	Dist. 3	Dist. 4	Dist. 5	Dist. 6	Dist. 7	Dist. 8
Step No	Bar	KN	mm	mm	mm	mm	mm	mm	mm	mm	mm
0.000	0.0	0.000	0.00	20.00	20.00		0.000	0.000		0.000	
1.000	2.1	0.702	0.01	19.97	18.93		0.030	0.070		0.050	
2.000	17.1	5.652	0.08	19.53	19.51		0.460	0.490		0.478	
3.000	24.2	11.304	0.16	19.41	19.24		0.590	0.760		0.673	
4.000	55.2	17.663	0.25	19.76	18.95		0.620	1.030		0.835	
5.000	70.5	23.315	0.31	19.36	18.76		0.910	1.240		0.940	
6.000	89.8	29.673	0.42	19.26	18.51		0.740	1.190		1.115	
7.000	106.4	35.325	0.50	19.21	18.32		0.790	1.680		1.235	
8.000	53.4	17.663	0.25	19.22	18.37		0.780	1.630		1.205	
9.000	26.7	8.831	0.12	19.34	18.39		0.750	1.610		1.185	
10.000	2.1	0.707	0.01	19.36	18.57		0.640	1.430		1.035	
11.000	2.1	0.707	0.01	19.34	18.57		0.640	1.430		1.035	
12.000	17.1	5.652	0.08	19.35	18.56		0.650	1.440		1.045	
13.000	34.2	11.304	0.16	19.34	18.52		0.650	1.480		1.070	
14.000	53.3	17.663	0.25	19.21	18.42		0.690	1.580		1.135	
15.000	70.5	23.315	0.31	19.26	18.30		0.720	1.700		1.210	
16.000	89.8	29.673	0.42	19.21	18.25		0.780	1.750		1.270	

		E_1	ΔS	Δs
0.7 m	0.35	1.01	0.26	0.2
0.3 m	0.15	0.05		
0.7 m	0.25	1.2233	0.0833	0.2
0.3 m	0.15	1.055		
D (mm)	300			
E_v	123.00			
E_{vp}	287.33			
Area (sqm)	0.07065			

Dist. 1	2.11		
---------	------	--	--

$$E_v = \frac{P}{\Delta s} \cdot \frac{D}{\Delta s} \cdot \frac{1}{A}$$

E_v = deformation modulus

Δs = load increment

D = settlement increment

D = diameter of the plate, generally 0.30 m

For this calculation Δs and D are usually taken from the load steps between 0.5 σ_{lim} and 0.7 σ_{lim} .

mm/mm

Sett.

Lab. Specialist

Name :

Sign :

Lab. Engineer

Name : AHMED HALEEM

Sign :

Consultant Engineer

Name : Farouk Ragab

Sign :



الهيئة العامة للطرق والمواصلات
General Authority of Roads and Transport

Contract Classification	CENTRAL LAB		Contracting	Client
Plate Load Test Results				
Company Name	AL MOSTAFA			
Location	524 + 760	To	524 + 900	
Test Date	28-09-2023			
Layer level	SUB BALLAST +0.90			

EQUIPMENT AND TEST PROCEDURE :

The diameter D of the plate is generally 0.30 m. For very coarse grained material also plates with diameter $D = 0.60$ m and $D = 0.182$ m are used.

The load is applied in 8 load increments of equal size. Under each load step the settlement must come to a noticeable end (> 0.02 mm/minute). After the maximum load is reached the unloading procedure can begin. After that, the plate is reloaded in 5 steps. A loaded truck, an excavator or a roller usually serve as counterweights for the hydraulic jack.

Diameter = 300mm

Loading	Load	Sett	Stress	Dist 1	Dist 2	Dist 3	Sett. 1	Sett. 2	Sett. 3	Avg. Sett.
Stage No.	Bar	mm	kN/m ²	mm	mm	mm	mm	mm	mm	mm
0.000	0.0	0.000	0.00	20.80	20.80		0.000	0.000		0.000
1.000	2.1	0.707	0.01	19.90	19.91		-0.100	0.040		0.070
2.000	17.1	5.652	0.08	18.70	18.80		0.300	0.200		0.250
3.000	34.2	11.304	0.16	18.30	19.70		0.700	0.300		0.500
4.000	51.3	17.653	0.25	18.03	19.58		0.970	0.480		0.895
5.000	70.5	23.315	0.33	18.61	19.40		1.190	0.600		0.895
6.000	89.8	29.673	0.42	18.45	19.22		1.350	0.780		1.065
7.000	106.8	35.329	0.50	18.41	18.05		1.590	0.950		1.270
8.000	124.8	41.683	0.58	18.43	19.08		1.550	0.920		1.235
9.000	142.7	48.031	0.67	18.54	18.13		1.480	0.870		1.165
10.000	160.6	54.379	0.76	18.71	18.22		1.290	0.780		1.035
11.000	178.5	60.727	0.85	18.71	18.22		1.290	0.780		1.035
12.000	196.4	67.075	0.94	18.70	18.21		1.300	0.790		1.045
13.000	214.3	73.423	1.03	18.62	19.18		1.280	0.840		1.110
14.000	232.2	79.771	1.12	18.50	19.11		1.500	0.890		1.195
15.000	250.1	86.119	1.21	18.43	19.08		1.570	0.920		1.245
16.000	268.0	92.467	1.30	18.39	19.03		1.620	0.970		1.295

	1	35	60
0.7 σ_{1s}	0.35	0.00343	0.11688
0.3 σ_{1s}	0.15	0.00275	
0.7 σ_{2s}	0.35	1.25611	0.29112
0.3 σ_{2s}	0.15	1.033	
D (mm)	300		
E_s	107.85		
E_{2s}	223.78		
Area (mm ²)	0.00071		

End Test	4.01		
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$$E_s = 0.75 - D \cdot \sigma_{1s} / \Delta s$$

E_s = deformation modulus

σ_{1s} = load increment

Δs = settlement increment

D = diameter of the plate, generally 0.30 m

For this calculation σ_{1s} and Δs are usually taken from the load span between 0.3 σ_{max} and 0.7 σ_{max} .

MM/M²

Sett.

Lab. Specialist

Name :

Sign :

Lab. Engineer

Name : AHMED HALEEM

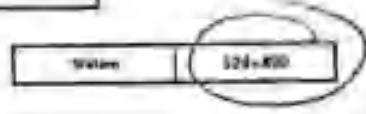
Sign :

Committee Engineer

Name : Youssef R. S. G.

Sign :

Client Consultant	Contractor/Consultant	CENTRAL LAB	Inspector	Owner
Plate Load Test Results				
Company Name	AL MOSTAFA			
Location	524 + 760	To	524 + 900	
Test Date	28-09-2023			
Layer level	SUB BALLAST +0.90			



EQUIPMENT AND TEST PROCEDURE :

The diameter D of the plate is generally 0.30 m. For very coarse grained material also plates with diameter $D = 0.50$ m and $D = 0.762$ m are used.

The load is applied in 6 load increments of equal size. Under each load step the settlement must come to a noticeable end (> 0.02 mm/minute). After the maximum load is reached the unloading procedure can begin. After that, the plate is reloaded in 5 steps. A loaded truck, an excavator or a roller usually serve as counterweight for the hydraulic jack.

Diameter = 300mm

Loading	Load	Load	Temp.	Dist 1	Dist 2	Dist 3	Sett 1	Sett 2	Sett 3	Sett
Stage No	Bar	PS	MM/100	mm	mm	mm	mm	mm	mm	mm
0.000	0.0	0.000	0.00	20.90	20.00		0.000	0.000		0.000
1.000	2.1	0.207	0.01	19.87	19.83		0.030	0.170		0.100
2.000	17.1	1.652	0.08	19.70	19.54		0.300	0.420		0.360
3.000	34.2	11.304	0.16	19.61	19.38		0.390	0.610		0.500
4.000	51.3	17.663	0.25	19.50	19.08		0.500	0.920		0.710
5.000	70.5	23.315	0.31	19.40	18.87		0.600	1.130		0.865
6.000	89.8	29.673	0.42	18.27	18.75		0.730	1.230		0.990
7.000	108.8	33.323	0.50	18.12	18.52		0.880	1.460		1.180
8.000	53.4	17.663	0.25	19.14	18.54		0.840	1.460		1.150
9.000	26.7	8.831	0.12	18.19	18.64		0.910	1.360		1.085
10.000	2.1	0.207	0.01	18.30	18.80		0.700	1.200		0.950
11.000	2.1	0.207	0.01	19.30	18.80		0.700	1.200		0.950
12.000	17.1	1.652	0.08	19.28	18.75		0.720	1.210		0.965
13.000	34.2	11.304	0.16	19.24	18.70		0.760	1.300		1.030
14.000	51.3	17.663	0.25	18.22	18.60		0.780	1.400		1.090
15.000	70.5	23.315	0.31	18.20	18.50		0.800	1.500		1.150
16.000	89.8	29.673	0.42	18.12	18.43		0.880	1.570		1.225

		AS	AS
0.7 σ_1	6.35	0.82373	0.38125
0.3 σ_1	6.15	0.4825	
0.5 σ_2	6.25	1.16087	0.18667
0.3 σ_2	6.15	0.38	
D (mm)	300		
E_v	131.87		
E_{v2}	241.97		
Area (sq.m)	0.07065		

σ_1/σ_2	1.01		
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$$E_v = 0.75 \cdot D \cdot \Delta \sigma / \Delta s$$

- E_v = deformation modulus
- D = load increment
- Δs = settlement increment
- D = diameter of the plate, generally 0.30 m

For this calculation, the settlement is usually taken from the load step between $D_1 = 0.50$ and 0.75 m.

WAM/PS1

Lab. Specialist Name : Sign :	Lab. Engineer Name : AHMED HALEEM Sign :	Consultant Engineer Name : Sign :
--	---	--

Owner Consultant	Contractor Consultant	CENTRAL LAB	Calculated by	Checked by
Plate Load Test Results				
Company Name	AL MOSTAFA			
Location	524 + 760	To	524 + 900	
Test Date	1-10-2023			
Layer level	SUB BALLAST +0.00			

Name

Signature

EQUIPMENT AND TEST PROCEDURE :-

The diameter D of the plate is generally 0.30 m. For very coarse grained material also plates with diameter $D = 0.60$ m and $D = 0.362$ m are used.

The load is applied in 8 load increments of equal size. Under each load step the settlement must come to a noticeable amt (≥ 0.02 mm/min). After the maximum load is reached the unloading procedure can begin. After that, the plate is refilled in 5 steps. A loaded truck, an excavator or a roller usually serve as counterweight for the hydraulic jack.

Diameter = 300mm

Loading Stage No.	Load P_n (kN)	Settle S_n (mm)	Dist 1' (mm)	Dist 2' (mm)	Dist 3' (mm)	Sett. 1' (mm)	Sett. 2' (mm)	Sett. 3' (mm)	Sett. Total (mm)
0.000	0.0	0.000	0.00	20.00	20.00	0.000	0.000		0.000
1.000	2.1	0.707	0.01	19.87	19.87	0.130	0.130		0.130
2.000	17.1	5.652	0.08	19.63	19.72	0.270	0.280		0.325
3.000	34.2	11.304	0.16	19.46	19.57	0.540	0.430		0.485
4.000	51.3	17.663	0.25	19.30	19.42	0.700	0.560		0.640
5.000	70.5	23.315	0.33	19.14	19.25	0.860	0.750		0.805
6.000	89.6	29.673	0.42	19.02	19.14	0.980	0.860		0.920
7.000	108.8	35.325	0.50	18.90	19.01	1.100	0.990		1.045
8.000	127.9	41.663	0.58	18.92	19.05	1.080	0.950		1.015
9.000	147.0	48.021	0.67	19.00	19.18	1.000	0.920		0.910
10.000	166.1	54.379	0.75	19.10	19.30	0.900	0.700		0.800
11.000	185.2	60.737	0.83	19.10	19.30	0.800	0.700		0.800
12.000	204.3	67.095	0.92	19.09	19.29	0.910	0.710		0.810
13.000	223.4	73.453	1.00	19.06	19.15	1.000	0.850		0.925
14.000	242.5	79.811	1.08	18.96	19.09	1.040	0.910		0.975
15.000	261.6	86.169	1.17	18.90	19.03	1.100	0.970		1.035

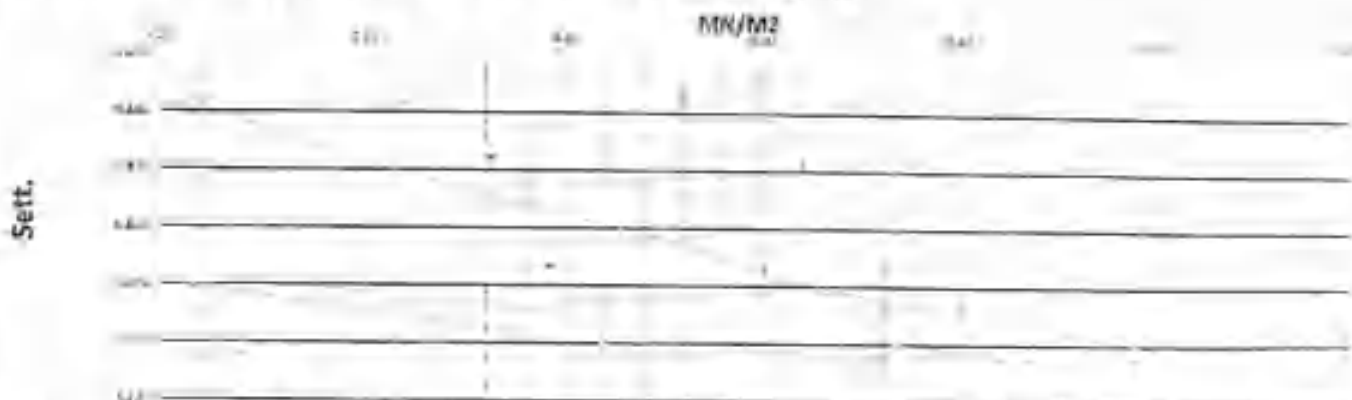
Load P_n (kN)	Sett. S_n (mm)	Dist 1' (mm)	Dist 2' (mm)	Dist 3' (mm)	Sett. Total (mm)
0.1 P_u	0.35	0.61063			0.35063
0.3 P_u	0.15	0.401			0.2
0.7 P_u	0.35	0.88832			0.2
0.3 P_u	0.15	0.82			0.10012
D (mm)	300				
E_v	139125				
E_v	787.33				
Area (sq.m)	0.07065				

Settlement	2.46		
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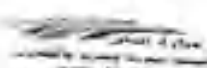
$$E_v = 0.75 \cdot D \cdot \Delta P / \Delta S$$

- E_v = deformation modulus
- ΔP = load increment
- ΔS = settlement increment
- D = diameter of the plate, generally 0.30 m

For this calculation ΔP and ΔS are usually taken from the load span between 0.3 P_{max} and 0.7 P_{max} .



Lab. Specialist	Lab. Engineer	Consultant Engineer
Name: _____	Name: AHMED HALEEM	Name: Youssef Rashed
Sign: _____	Sign:	Sign:



المجلس الوطني للتخطيط العمراني
Ministry of Public Works and Urban Planning

Contractor	Contractor Company	CENTRAL LAB	Completion	Date
Plate Load Test Results				
AL MOSTAFA				
Company Name				
Location	524 + 640	To	524 + 760	
Test Date	1-10-2023			
Layer level	SUB BALLAST +0.90			

EQUIPMENT AND TEST PROCEDURE :-

The diameter D of the plate is generally 0.30 m. For very coarse grained material also plates with diameter $D = 0.60$ m and $D = 0.762$ m are used.

The load is applied in 8 load increments of equal size. Under each load step the settlement must come to a noticeable end (< 0.02 mm/min). After the maximum load is reached the unloading procedure can begin. After that, the plate is reloaded in 5 steps. A loaded truck, an excavator or a roller usually serve as counterweight for the hydraulic jack.

Diameter = 300mm

Load	Load	Load	Settlement	Settlement	Settlement	Settlement	Settlement	Settlement	Settlement	Settlement
Step No.	Bar	kN	mm	mm	mm	mm	mm	mm	mm	mm
0.000	0.0	0.000	0.00	20.00	20.00		0.000	0.000		0.000
1.000	2.1	0.797	0.01	19.93	19.95		0.050	0.050		0.050
2.000	17.1	5.652	0.08	19.70	19.80		0.300	0.340		0.220
3.000	34.2	11.304	0.16	19.60	19.57		0.400	0.480		0.440
4.000	51.3	17.663	0.25	19.57	19.30		0.430	0.700		0.565
5.000	70.5	23.315	0.33	19.35	19.20		0.650	0.800		0.725
6.000	89.8	29.673	0.42	19.25	19.07		0.750	0.930		0.840
7.000	106.8	35.325	0.50	19.15	18.93		0.850	1.070		0.960
8.000	124.4	41.663	0.55	19.18	18.88		0.820	1.020		0.920
9.000	142.7	48.931	0.52	19.23	19.04		0.750	0.960		0.875
10.000	161.0	56.200	0.41	19.33	19.15		0.870	0.850		0.760
11.000	179.3	63.468	0.38	19.32	19.14		0.880	0.800		0.770
12.000	197.6	70.736	0.36	19.26	19.07		0.740	0.930		0.835
13.000	215.9	78.004	0.28	19.21	19.01		0.790	0.900		0.890
14.000	234.2	85.272	0.23	19.18	18.95		0.820	1.050		0.935
15.000	252.5	92.540	0.22	19.15	18.91		0.850	1.080		0.974

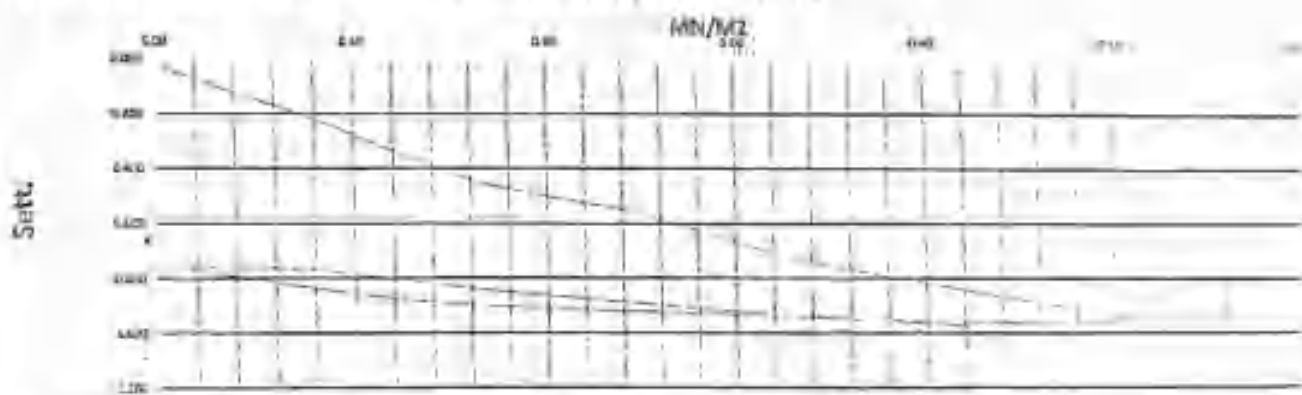
Step No.	Bar	kN	mm	mm	mm	mm	mm	mm	mm	mm
0.000	0.0	0.000	0.00	20.00	20.00		0.000	0.000		0.000
1.000	2.1	0.797	0.01	19.93	19.95		0.050	0.050		0.050
2.000	17.1	5.652	0.08	19.70	19.80		0.300	0.340		0.220
3.000	34.2	11.304	0.16	19.60	19.57		0.400	0.480		0.440
4.000	51.3	17.663	0.25	19.57	19.30		0.430	0.700		0.565
5.000	70.5	23.315	0.33	19.35	19.20		0.650	0.800		0.725
6.000	89.8	29.673	0.42	19.25	19.07		0.750	0.930		0.840
7.000	106.8	35.325	0.50	19.15	18.93		0.850	1.070		0.960
8.000	124.4	41.663	0.55	19.18	18.88		0.820	1.020		0.920
9.000	142.7	48.931	0.52	19.23	19.04		0.750	0.960		0.875
10.000	161.0	56.200	0.41	19.33	19.15		0.870	0.850		0.760
11.000	179.3	63.468	0.38	19.32	19.14		0.880	0.800		0.770
12.000	197.6	70.736	0.36	19.26	19.07		0.740	0.930		0.835
13.000	215.9	78.004	0.28	19.21	19.01		0.790	0.900		0.890
14.000	234.2	85.272	0.23	19.18	18.95		0.820	1.050		0.935
15.000	252.5	92.540	0.22	19.15	18.91		0.850	1.080		0.974

Settlement	1.00		
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$$E_s = 0.73 \cdot D \cdot \ln \left(\frac{D}{d} \right)$$

- E_s = deformation modulus
- D = load increment
- d = settlement increment
- D = diameter of the plate, generally 0.30 m

For this calculation d_1 and d_2 are usually taken from the load point between 0.3 kN and 0.7 kN.



Lab. Specialist	Lab. Engineer	Construction Engineer
Name:	Name: AHMED HALEEM	Name: Youssef Rashed
Sign:	Sign: [Signature]	Sign: [Signature]



MINISTRY OF TRANSPORT AND PUBLIC WORKS
GENERAL LABORATORY



GENERAL LABORATORY



GENERAL LABORATORY

GENERAL LABORATORY

Consultant Consultant

CENTRAL LAB

Engineering

Project

General

Plate Load Test Results

Company Name

AL MOSTAFA

Location

524 + 500

To

524 + 640

Test Date

28-09-2023

Layer level

SUB BALLAST +0.00

Station

374+871

EQUIPMENT AND TEST PROCEDURE :-

The diameter D of the plate is generally 0.30 m. For very coarse grained material also plates with diameter $D = 0.60$ m and $D = 0.75$ m are used.

The load is applied in 8 load increments of equal size. Under each load step the settlement must come to a noticeable end (≈ 0.02 mm/min). After the maximum load is reached the unloading procedure can begin. After that, the plate is reloaded in 5 steps. A loaded truck, an excavator or a roller usually serves as counterweight for the hydraulic jack.

Diameter = 300mm

Loadings	Load	Load	Settle	Load	Load	Load	Settle	Settle	Settle	Settle
Stage No.	Size	kN	kN/m ²	mm	mm	mm	mm	mm	mm	mm
0.000	0.0	0.000	0.00	20.00	20.00		0.000	0.000		0.000
1.000	2.1	0.707	0.01	19.04	19.07		0.020	0.030		0.025
2.000	17.1	5.652	0.04	19.04	19.75		0.120	0.250		0.185
3.000	34.2	11.304	0.10	19.75	19.56		0.250	0.440		0.345
4.000	51.3	17.083	0.25	19.64	19.34		0.300	0.600		0.510
5.000	70.5	23.315	0.33	19.44	19.24		0.540	0.760		0.660
6.000	88.8	28.973	0.42	19.34	19.07		0.660	0.930		0.795
7.000	108.8	35.325	0.50	19.20	18.80		0.800	1.100		0.950
8.000	134.4	43.663	0.75	19.20	19.03		0.740	0.970		0.855
9.000	20.7	6.831	0.12	19.31	19.10		0.600	0.900		0.795
10.000	2.1	0.707	0.01	19.46	19.30		0.520	0.700		0.610
11.000	2.1	0.707	0.01	19.18	19.30		0.520	0.700		0.610
12.000	17.1	5.652	0.08	19.44	19.28		0.540	0.740		0.630
13.000	34.2	11.304	0.16	19.33	19.29		0.670	0.800		0.735
14.000	51.3	17.083	0.25	19.28	19.13		0.720	0.870		0.795
15.000	70.5	23.315	0.33	19.24	19.05		0.780	0.950		0.855
16.000	88.8	28.973	0.42	19.18	19.00		0.820	1.000		0.910

	α	β	γ
0.2 σ_1	0.35	0.4073	0.34437
0.3 σ_1	0.14	0.311	
0.7 σ_1	0.15	0.4073	0.17732
0.3 σ_1	0.14	0.60	
D (mm)	300		
E_v	114.58		
E_v	253.82		
σ_{max} (kPa)	0.8780		

Excess	1.00		
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$$E_v = 0.73 \cdot D \cdot \sigma_1 / \sigma_2$$

E_v = deformation modulus

σ_1 = load increment

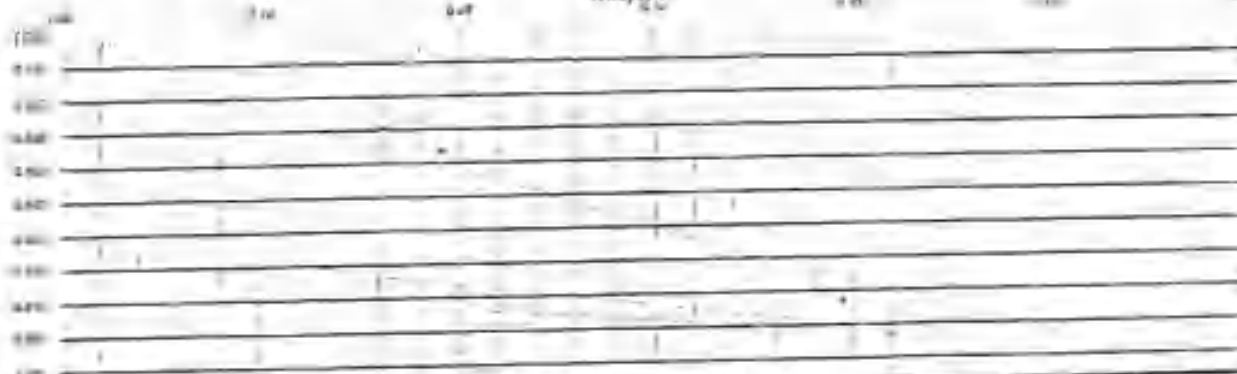
σ_2 = settlement increment

D = diameter of the plate, generally 0.30 m

For this calculation σ_1 and σ_2 are usually taken from the load step between 0.7 σ_{max} and 0.7 σ_{max} .

MM/M2

Sett.



Lab. Specialist

Name:

Sign:

Lab. Engineer

Name: AHMED HALEEM

Sign:

Consultant Engineer

Name: Youssef Rashed

Sign:

Plate Load Test Results

Company Name	AL MOSTAFA		
Location	524 + 500	To	524 + 640
Test Date	28-09-2023		
Layer level	SUB BALLAST +0.90		

Station

524+505

EQUIPMENT AND TEST PROCEDURE

The diameter D of the plate is generally 0.30 m. For very coarse grained material also plates with diameter $D = 0.60$ m and $D = 0.762$ m are used

The load is applied in 8 load increments of equal size. Under each load step the settlement must come to a noticeable end (< 0.02 mm/minute). After the maximum load is reached the unloading procedure can begin. After that, the plate is reloaded in 5 steps. A loaded truck, an excavator or a roller usually serve as counterweight for the hydraulic jack.

Diameter = 300mm

Loading	Test	Load	Area	Sett.	Diff. 1	Diff. 2	Sett. 1	Sett. 2	Sett. 3	Sett. 4
Stage No.	No.	kN	mm ²	mm	mm	mm	mm	mm	mm	mm
0.000	0.0	0.000	0.00	20.00	20.00		0.000	0.000		0.000
1.000	2.1	0.707	0.01	19.92	19.91		0.080	0.030		0.055
2.000	17.1	5.652	0.08	19.60	19.70		0.400	0.300		0.350
3.000	34.2	11.304	0.16	19.25	19.43		0.750	0.570		0.690
4.000	51.3	17.863	0.25	18.90	19.45		1.100	0.350		0.825
5.000	78.5	23.315	0.33	18.70	19.27		1.390	0.730		1.015
6.000	89.8	29.675	0.42	18.50	19.13		1.500	0.870		1.185
7.000	106.8	35.325	0.50	18.25	18.90		1.750	1.100		1.425
8.000	53.4	17.863	0.25	18.30	19.03		1.700	0.970		1.325
9.000	26.7	8.831	0.12	18.43	19.06		1.570	0.960		1.255
9.500	2.1	0.707	0.01	18.62	19.11		1.380	0.890		1.135
10.000	2.1	0.707	0.01	18.42	19.11		1.380	0.890		1.135
11.000	17.1	5.652	0.08	18.63	19.10		1.290	0.900		1.145
12.000	34.2	11.304	0.16	18.50	19.08		1.500	0.920		1.210
13.000	51.3	17.863	0.25	18.40	19.05		1.600	0.930		1.275
14.000	78.5	23.315	0.33	18.30	19.03		1.700	0.970		1.325
15.000	89.8	29.675	0.42	18.25	19.00		1.750	1.000		1.375

	α	β	γ
0.7 α_1	0.35	0.975	0.35375
0.3 α_2	0.15	0.82125	
0.7 α_3	0.35	1.34389	0.18895
0.3 α_4	0.15	1.133	
D (mm)	300		
E_{v1}	127.21		
E_{v2}	238.24		
Area (sq.m)	0.07065		

Sett. 1	1.01		
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$$E_v = \frac{d \cdot Q}{D \cdot \Delta s} \cdot \frac{d \cdot r}{d \cdot r}$$

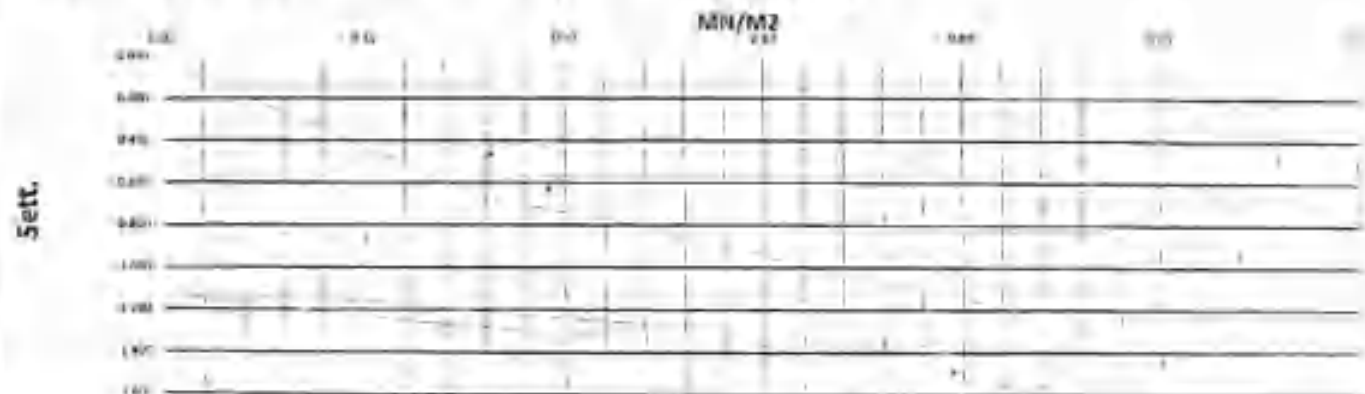
E_v = deformation modulus

Q = load increment

Δs = settlement increment

D = diameter of the plate, generally 0.30 m

For this calculation Δs and α are usually taken from the load span between 0.3 α_{max} and 0.7 α_{max}




Lab Specialist

Name: _____

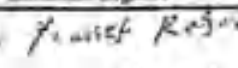
Sign: _____

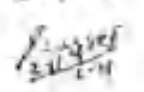
Lab Engineer

Name: AHMED HALEEM

Sign: 

Consultant Engineer

Name: 

Sign: 



Project Consultant	Contractor Consultant	CENTRAL LAB	Company	Station
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Plate Load Test Results			
Company Name	AL MOSTAFA		
Location	524 + 500	To	524 + 640
Test Date	1-10-2023		
Layer level	SUB BALLAST +0.90		

Station	524+575
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EQUIPMENT AND TEST PROCEDURE :-

The diameter D of the plate is generally 0.30 m. For very coarse grained material also plates with diameter $D = 0.60$ m and $D = 0.762$ m are used.

The load is applied in 8 load increments of equal size. Under each load step the settlement must come to a noticeable end (≤ 0.02 mm/minute). After the maximum load is reached the unloading procedure can begin. After this, the plate is reloaded in 5 steps. A loaded truck, an excavator or a roller usually serve as counterweights for the hydraulic jack.

Diameter = 300mm

Load Step	Load (kN)	Settlement (mm)	Settlement (mm)	Settlement (mm)	Settlement (mm)	Settlement (mm)	Settlement (mm)	Settlement (mm)	Settlement (mm)	Settlement (mm)	Settlement (mm)
0.000	0.0	0.000	0.00	20.00	20.00		0.000	0.000		0.000	
1.000	2.1	0.707	0.01	19.80	19.87		0.200	0.130		0.165	
2.000	17.1	3.652	0.08	19.43	19.52		0.570	0.480		0.525	
3.000	34.2	11.304	0.16	19.21	19.31		0.790	0.690		0.740	
4.000	51.3	17.663	0.25	19.07	19.21		0.930	0.790		0.860	
5.000	70.5	23.315	0.33	18.98	19.08		1.100	0.920		1.010	
6.000	89.8	29.673	0.42	18.81	18.99		1.190	1.010		1.100	
7.000	106.8	35.323	0.50	18.62	18.80		1.400	1.200		1.300	
8.000	121.4	42.663	0.55	18.65	18.83		1.350	1.170		1.260	
9.000	26.7	8.931	0.12	18.81	18.89		1.090	1.110		1.130	
10.000	2.1	0.707	0.01	18.95	18.97		1.030	1.020		1.040	
11.000	2.1	0.707	0.01	18.95	18.97		1.050	1.030		1.040	
12.000	17.1	3.652	0.08	18.91	18.94		1.090	1.060		1.075	
13.000	34.2	11.304	0.16	18.83	18.92		1.170	1.080		1.123	
14.000	51.3	17.663	0.25	18.74	18.88		1.250	1.120		1.190	
15.000	70.5	23.315	0.33	18.68	18.85		1.320	1.150		1.235	
16.000	89.8	29.673	0.42	18.62	18.82		1.380	1.180		1.280	

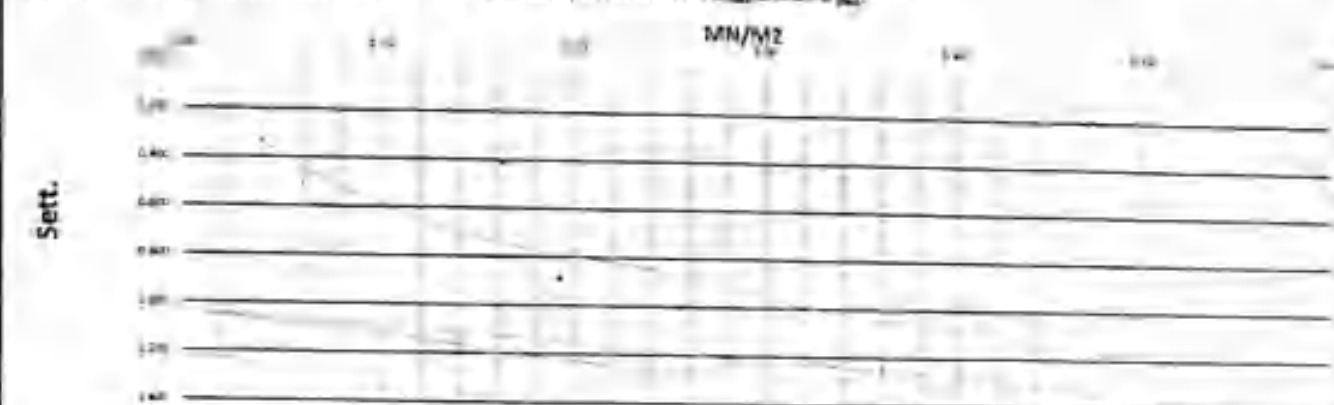
Load Step	Load (kN)	Settlement (mm)	Settlement (mm)	Settlement (mm)	Settlement (mm)
0.7 σ_c	0.35	0.925	0.23186	0.2	
0.3 σ_c	0.15	0.71313			
0.7 σ_c	0.35	0.925	0.135	0.2	
0.3 σ_c	0.15	0.713			
D (mm)	300				
E_v	211.35				
E_v	333.34				
Area (kg/cm²)	0.0708				

Factor	1.07		
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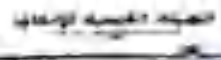
$$E_v = 0.13 \cdot D \cdot \ln \left(\frac{D}{d} \right)$$

- E_v = deformation modulus
- D_v = load increment
- D_s = settlement increment
- D = diameter of the plate, generally 0.30 m

For this calculation d and \ln are usually taken from the load step between 0.3 $\sigma_{c, max}$ and 0.7 $\sigma_{c, max}$.



Lab. Specialist	Lab. Engineer	Consultant Engineer
Name: _____	Name: AHMED HALEEM	Name: Jassim R-B
Sign: _____	Sign:	Sign:



Contractor/Consultant	CENTRAL LAB	Demolisher	None
Plate Load Test Results:			
Company Name	AL MOSTAPA		
Location	524 + 500	To	524 + 640
Test Date	1-10-2023		
Layer level	SUB BALLAST +0.90		

Station: 524+553

EQUIPMENT AND TEST PROCEDURE :-

The diameter D of the plate is generally 0.30 m. For very coarse grained material also plates with diameter $D = 0.60$ m and $D = 0.762$ m are used.

The load is applied in 8 load increments of equal size. Under each load, stop the settlement must come to a noticeable end (> 0.02 mm/min). After the maximum load is reached the unloading procedure can begin. After that, the plate is reloaded in 4 steps. A loaded truck, an excavator or a roller usually serve as counterweight for the hydraulic jack.

Diameter: 300mm

Load No	Load (kN)	Load (ton)	Stress (kN/m ²)	Sett (mm)	Rate (mm/min)	Final (mm)	SPL 1 (mm)	SPL 2 (mm)	Sett. 3 (mm)	Avg. Sett. (mm)
0.000	0.0	0.000	0.00	20.00	20.00		0.000	0.000		0.000
1.000	2.3	0.707	0.01	19.95	19.95		0.050	0.050		0.050
2.000	17.1	5.352	0.08	19.85	19.86		0.150	0.140		0.145
3.000	34.2	11.304	0.16	19.65	19.80		0.350	0.310		0.330
4.000	52.3	17.603	0.25	19.51	19.62		0.490	0.460		0.485
5.000	70.5	23.915	0.33	19.36	19.40		0.640	0.600		0.620
6.000	89.8	29.673	0.42	19.23	19.30		0.770	0.700		0.735
7.000	108.8	35.325	0.50	19.10	19.11		0.900	0.890		0.895
8.000	127.8	41.063	0.58	19.15	19.15		0.950	0.950		0.950
9.000	146.7	46.831	0.67	19.20	19.25		0.980	0.750		0.875
10.000	165.7	52.515	0.76	19.28	19.34		0.710	0.660		0.685
11.000	184.7	58.200	0.85	19.29	19.34		0.730	0.670		0.695
12.000	203.7	63.885	0.94	19.23	19.28		0.770	0.720		0.745
13.000	222.7	69.569	1.03	19.18	19.25		0.820	0.750		0.785
14.000	241.7	75.254	1.12	19.15	19.20		0.850	0.800		0.825
15.000	260.7	80.938	1.21	19.10	19.12		0.900	0.880		0.890

	1	2	3
0.1 m	0.35	0.595	0.29010
0.3 m	0.15	0.30000	0.2
0.7 m	0.35	0.30000	0.2
0.7 m	0.15	0.300	
0.7 m	0.00		
E_p	158.18		
E_v	324.21		
Area (kg/m ²)	9.8100		

E_v/E_p	2.14		
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$$E_p = 0.22 \cdot D \cdot \sigma_p / \delta_p$$

δ_p = deformation measured

D_p = load measured

δ_p = settlement measured

D = diameter of the plate, generally 0.30 m

For this calculation δ_p and δ_v are usually taken from the load span between 0.1 σ_{max} and 0.7 σ_{max} .



Lab. Specialist

Name:

Sign:

Lab. Engineer

Name:

Sign:

AMMEDI HALEEM

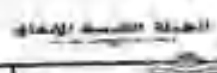
Consultant Engineer

Name:

Sign:

Amr Rashed

Amr Rashed



Client Organization	Contract Organization	CENTRAL LAB	Contractor	Open
Plate Load Test Results				
Company Name	AL MOSTAFA			
Location	524 + 500	To	524 + 840	Scale
Test Date	1-10-2023			
Layer level	SUB BALLAST +0.90			

EQUIPMENT AND TEST PROCEDURE :-

The diameter D of the plate is generally 0.30 m. For very coarse grained material also plates with diameter $D = 0.60$ m and $D = 0.762$ m are used.

The load is applied in 6 load increments of equal size. Under each load step the settlement must come to a noticeable end (≈ 0.02 mm/minute). After the maximum load is reached the unloading procedure can begin. After that, the plate is reloaded in 5 steps. A loaded truck, an excavator or a roller usually serve as counterweight for the hydraulic jack.

Diameter = 300mm

Load	Load	Settle	Total	Dist 1	Dist 2	Dist 3	Sett. 1	Sett. 2	Sett. 3	Sett. 4
Step No.	Bar	KN	mm	mm	mm	mm	mm	mm	mm	mm
0.000	0.0	0.000	0.00	20.00	20.00		0.000	0.000		0.000
1.000	2.1	0.707	0.01	19.89	19.89		0.010	0.010		0.010
2.000	17.1	5.652	0.08	19.72	19.75		0.280	0.250		0.265
3.000	34.2	11.304	0.16	19.57	19.27		0.630	0.730		0.680
4.000	51.3	17.663	0.25	19.35	19.11		0.850	0.890		0.870
5.000	70.5	23.313	0.33	19.02	18.83		0.970	1.150		1.060
6.000	89.8	29.673	0.42	18.85	18.65		1.150	1.350		1.250
7.000	109.0	35.325	0.50	18.60	18.50		1.400	1.500		1.450
8.000	128.2	41.680	0.58	18.72	18.51		1.280	1.450		1.265
9.000	147.4	48.031	0.62	18.78	18.60		1.210	1.400		1.305
10.000	166.6	54.382	0.61	18.90	18.75		1.100	1.250		1.175
11.000	185.8	60.733	0.61	18.90	18.75		1.100	1.250		1.175
12.000	205.0	67.084	0.60	18.89	18.74		1.110	1.260		1.185
13.000	224.2	73.435	0.58	18.85	18.69		1.150	1.310		1.230
14.000	243.4	79.786	0.55	18.79	18.60		1.210	1.400		1.305
15.000	262.6	86.137	0.52	18.72	18.52		1.290	1.480		1.380
16.000	281.8	92.488	0.42	18.60	18.40		1.400	1.600		1.500

	σ	ΔS	ΔS
0.7 σ_{max}	0.35	1.075	0.44666
0.5 σ_{max}	0.15	0.62833	
0.7 σ_{min}	0.35	1.40667	0.21167
0.5 σ_{min}	0.15	1.195	
D (mm)	300		
E_s	100.70		
E_v	212.80		
Area (sq. m)	0.07068		

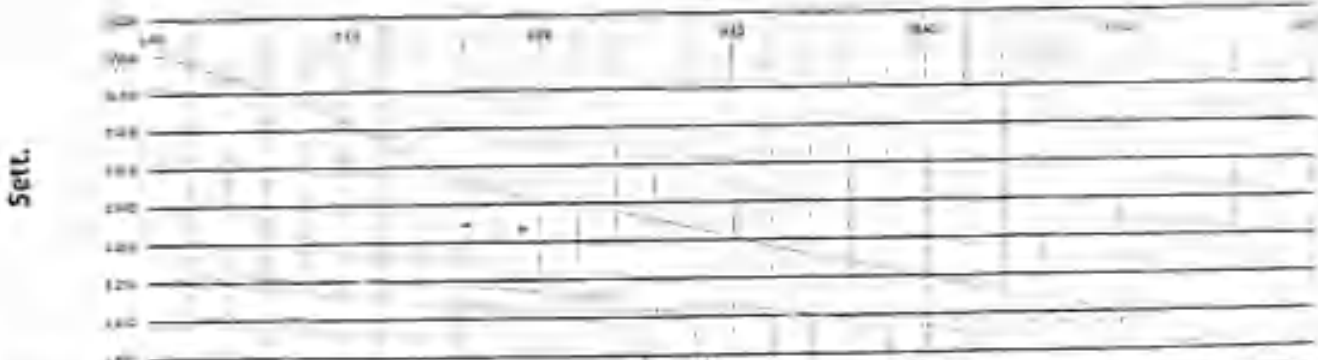
Settlement	2.51		
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$$E_s = 0.75 \cdot D \cdot \Delta S / \Delta S$$





- E_s = deformation modulus
- ΔS = load increment
- D = settlement increment
- D = diameter of the plate, generally 0.30 m

For this calculation ΔS and ΔS are usually taken from the load span between 0.1 σ_{max} and 0.7 σ_{max}

MN/M2



Lab. Specialist	Lab. Engineer	Consultant Engineer
Name: _____	Name: AHMED NALEEM	Name: Fawaz Khatib
Sign: _____	Sign: _____	Sign: _____

 (Ministry of Transport and Public Works)	 (Ministry of Transport and Public Works)	 (Ministry of Transport and Public Works)	 (Ministry of Transport and Public Works)	 (Ministry of Transport and Public Works)
(Project Consultant)	Contractor (Licensed)	CENTRAL LAB	Contractor	Name
Plate Load Test Results				
Company Name		AL MOSTAFA		
Location	524 + 610	To	524 + 700	
Test Date	28-09-2023			
Layer level	SUB BALLAST +0.90			
EQUIPMENT AND TEST PROCEDURE :				

The diameter D of the plate is generally 0.30 m. For very coarse grained material also plates with diameter $D = 0.40$ m and $D = 0.75$ m are used.

The load is applied in 8 load increments of equal size. Under each load step the settlement must come to a noticeable and (≥ 0.02 mm/second). After the maximum load is reached the unloading procedure can begin. After that, the plate is reloaded in 5 steps. A loaded truck, an excavator or a roller usually serve as counterweight for the hydraulic jack.

Diameter = 300mm

Loading	Load	Load	Stress	Dist 1	Dist 2	Dist 3	Sett. 1	Sett. 2	Sett. 3	Avg. Sett.
Stage No	Bar	kN	kN/m ²	mm	mm	mm	mm	mm	mm	mm
0.000	0.0	0.000	0.00	20.00	20.00		0.000	0.000		0.000
1.000	2.1	0.707	0.01	19.96	19.95		0.040	0.050		0.045
2.000	17.1	5.652	0.08	19.90	19.64		0.100	0.360		0.230
3.000	34.2	11.304	0.16	19.80	19.32		0.200	0.680		0.440
4.000	51.3	17.663	0.25	19.75	19.07		0.250	0.930		0.590
5.000	70.5	23.315	0.33	19.70	18.80		0.300	1.200		0.750
6.000	89.8	29.673	0.42	19.61	18.60		0.390	1.400		0.895
7.000	109.8	35.325	0.50	19.50	18.43		0.500	1.570		1.035
8.000	129.4	41.663	0.55	19.03	18.58		0.570	1.420		0.895
9.000	149.7	48.831	0.62	19.68	18.73		0.320	1.290		0.805
10.000	169.7	55.875	0.69	19.72	18.96		0.280	1.020		0.650
11.000	189.7	62.707	0.75	19.72	18.98		0.280	1.020		0.650
12.000	209.7	69.325	0.81	19.60	18.81		0.360	1.190		0.775
13.000	229.7	75.831	0.87	19.60	18.73		0.400	1.270		0.835
14.000	249.7	82.163	0.93	19.57	18.59		0.430	1.410		0.920
15.000	269.7	88.325	0.99	19.53	18.45		0.470	1.550		1.010

	σ	Δs	Δs	Δs
0.7 σ_1	0.25	0.7753	0.52873	0.2
0.3 σ_2	0.15	0.9373		
0.7 σ_3	0.25	0.94	0.21	0.2
0.3 σ_4	0.15	0.73		
D (mm)	300			
E_{p1}	125.44			
E_{p2}	214.28			
Area (Sq.m)	0.07068			

Modulus	(%)		
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$$E_p = 0.71 \cdot D \cdot \Delta s / \Delta s$$

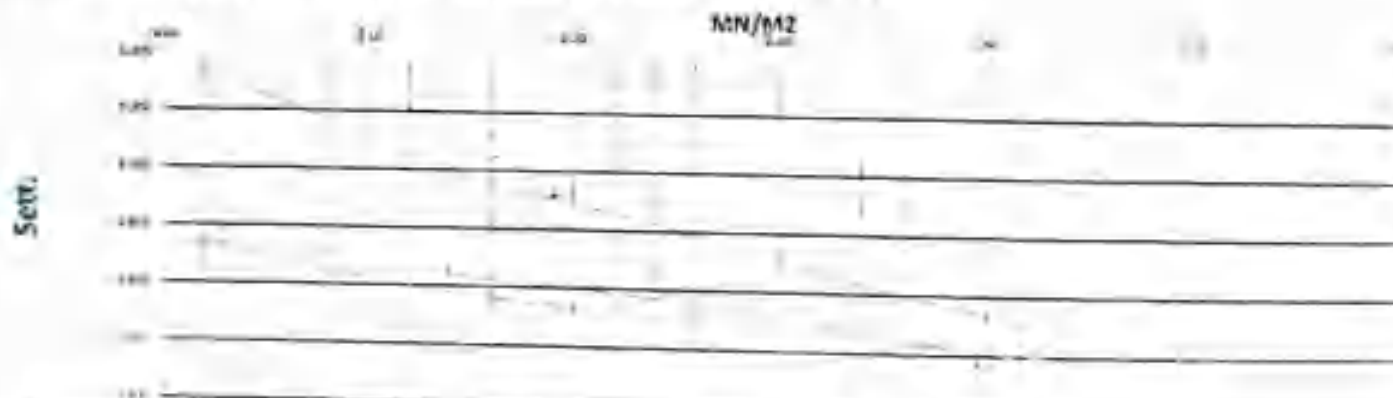
E_p = deformation modulus

Δs = load increment

Δs = settlement increment

D = diameter of the plate, generally 0.30 m

For this calculation Δs_1 and Δs_2 are usually taken from the load span between 0.3 σ_{max} and 0.7 σ_{max} .



Lab Specialist

Name :

Sign :

Lab Engineer

Name : AHMED HALEEM

Sign :

Consultant Engineer

Name : /

Sign :



Owner Committee

Contractor Committee

CENTRAL LAB

Contractor

Drawing

Plate Load Test Results

Company Name	AL MOSTAFA		
Location	524 + 640	To	524 + 760
Test Date	1-10-2023		
Layer level	SUB BALLAST +0.90		

Station	524+733
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EQUIPMENT AND TEST PROCEDURE

The diameter D of the plate is generally 0.30 m. For very coarse grained material also plates with diameter $D = 0.60$ m and $D = 0.762$ m are used.

The load is applied in 6 load increments of equal size. Under each load step the settlement must come to a noticeable end (< 0.02 mm/minute). After the maximum load is reached the unloading procedure can begin. After that, the plate is reloaded in 5 steps. A loaded truck, an excavator or a roller usually serve as counterweight for the hydraulic jack.

Diameter = 300mm

Loading Stage No.	Load kN	Load kN	Stress MPa	Sett. mm	Sett. mm	Sett. mm	Sett. mm	Sett. mm	Sett. mm	Avg Sett. mm
0.000	0.0	0.000	0.00	20.00	20.00		0.000	0.000		0.000
1.000	2.1	0.707	0.01	19.90	19.97		0.010	0.030		0.020
2.000	17.1	5.652	0.08	19.97	19.94		0.030	0.060		0.045
3.000	34.2	11.304	0.16	19.80	19.80		0.120	0.120		0.120
4.000	51.3	17.061	0.25	19.80	19.82		0.200	0.180		0.190
5.000	70.5	23.115	0.33	19.72	19.74		0.260	0.260		0.270
6.000	89.8	29.673	0.42	19.58	19.68		0.410	0.320		0.365
7.000	106.8	35.315	0.50	19.51	19.60		0.490	0.400		0.445
8.000	51.3	17.061	0.25	19.56	19.64		0.440	0.360		0.400
9.000	24.7	8.031	0.12	19.68	19.71		0.360	0.280		0.325
10.000	2.1	0.707	0.01	19.78	19.85		0.220	0.150		0.185
11.000	2.1	0.707	0.01	19.78	19.85		0.220	0.150		0.185
12.000	17.1	5.652	0.08	19.70	19.83		0.300	0.170		0.235
13.000	34.2	11.304	0.16	19.65	19.81		0.350	0.190		0.270
14.000	51.3	17.061	0.25	19.60	19.76		0.400	0.240		0.320
15.000	70.5	23.115	0.33	19.50	19.71		0.500	0.290		0.395
16.000	89.8	29.673	0.42	19.45	19.61		0.550	0.390		0.470

	σ	Δs	Δs
0.7 σ_1	0.33	0.211	0.18438
0.5 σ_1	0.15	0.11061	
0.2 σ_1	0.05	0.41187	
0.1 σ_1	0.01	0.28501	0.12665
D (mm)	300		
E_v	244.87		
E_v	121.28		
Area / σ_{max}	0.0786		

Sett. (mm)	1.18		
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$$E_v = 0.73 \cdot D \cdot \Delta s / \Delta \sigma$$

- E_v = deformation modulus
- $\Delta \sigma$ = load increment
- Δs = settlement increment
- D = diameter of the plate, generally 0.30 m

For this calculation $\Delta \sigma$ and Δs are usually taken from the load span between 0.5 σ_{max} and 0.7 σ_{max} .

MN/M2

Sett.

Lab. Specialist

Lab. Engineer

Consultant Engineer

Name:

Name:

AHMED HALEEM

Name:

Y. M. F. R. 2023

Sign:

Sign:

Sign:

Plate Load Test Results			
Company Name		AL MOSTAFA	
Location	524 + 640	To	524 + 760
Test Date	28-09-2023		
Layer level	SUB BALLAST +0.00		

Project
S04-715

EQUIPMENT AND TEST PROCEDURE :-

The diameter D of the plate is generally 0.30 m. For very coarse grained material also plates with diameter $D = 0.60$ m and $D = 0.762$ m are used.

The load is applied in 6 load increments of equal size. Under each load step the settlement must come to a noticeable end (≤ 0.02 mm/minute). After the maximum load is reached the unloading procedure can begin. After that, the plate is reloaded in 5 steps. A loaded truck, an excavator or a roller usually serves as counterweight for the hydraulic jack.

Diameter = 300mm

Load Step	Load	Load	Stress	Total	Total	Total	Sett	Sett	Sett	Sett
Step No	Bar	kN	N/mm ²	mm	mm	mm	mm	mm	mm	mm
0.000	0.0	0.000	0.00	20.00	20.00		0.000	0.000		0.000
1.000	2.1	0.707	0.01	19.98	19.97		0.020	0.020		0.020
2.000	17.1	5.652	0.08	19.75	19.50		0.250	0.500		0.375
3.000	24.2	11.301	0.16	19.62	19.15		0.380	0.850		0.615
4.000	53.3	17.603	0.25	19.32	18.92		0.480	1.080		0.780
5.000	70.5	23.315	0.33	19.43	18.60		0.550	1.400		0.875
6.000	89.8	29.623	0.42	19.37	18.46		0.630	1.560		1.083
7.000	106.8	35.325	0.50	19.20	18.38		0.800	1.760		1.250
8.000	53.3	17.603	0.25	19.28	18.33		0.710	1.670		1.190
9.000	26.7	8.811	0.12	19.43	18.37		0.570	1.630		1.100
9.000	2.1	0.707	0.01	19.51	18.38		0.470	1.410		0.940
10.000	2.1	0.707	0.01	19.53	18.59		0.470	1.410		0.940
11.000	17.1	5.652	0.08	19.32	19.52		0.480	1.470		0.975
12.000	34.2	11.304	0.16	19.48	18.46		0.520	1.540		1.050
13.000	53.3	17.603	0.25	19.44	18.32		0.560	1.680		1.120
14.000	70.5	23.315	0.33	19.40	18.25		0.600	1.750		1.175
15.000	89.8	29.623	0.42	19.32	18.18		0.680	1.820		1.250

0.7 σ_1	0.35	0.94002	0.35502	0.2
0.3 σ_1	0.15	0.380		
0.7 σ_2	0.35	1.19187	0.16180	0.2
0.3 σ_2	0.15	1.01		
D (mm)	300			
E_{vj}	125.54			
E_{vj}	247.71			
Area (Sq.m)	0.0900			

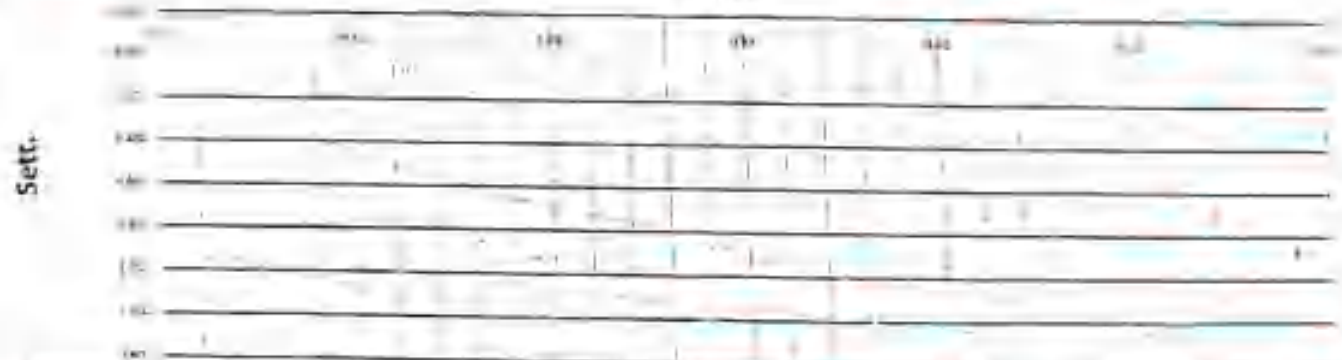
Settlement
1.88

$E_p = 0.75 \cdot D \cdot \sigma_1 / \delta_1$

- E_p = deformation modulus
- σ_1 = load increment
- δ_1 = settlement increment
- D = diameter of the plate; generally 0.30 m

For this calculation σ_1 and δ_1 are usually taken from the load span between 0.3 σ_{max} and 0.7 σ_{max} .

kN/m²



Lab. Specialist

Name :

Sign :

Lab. Engineer

Name : AHMED HALEEM

Sign : *[Signature]*

Consultant Engineer

Name : *[Signature]*

Sign : *[Signature]*

Owner Committee	Contractor Committee	CENTRAL LAB	Contractor	Times
Plate Load Test Results				
Company Name		AL MOSTAFA		
Location	524 + 640	To	524 + 760	
Test Date	1-10-2023			
Layer level	SUB BALLAST +0.90			

Station: 524+895

EQUIPMENT AND TEST PROCEDURE :-

The diameter, D of the plate is generally 0.30 m. For very coarse grained material also plates with diameter $D = 0.80$ m and $D = 0.762$ m are used

The load is applied in 8 load increments of equal size. Under each load step the settlement must come to a noticeable end (≤ 0.02 mm/minute). After the maximum load is reached the unloading procedure can begin. After that, the plate is reloaded in 5 steps. A loaded truck, an excavator or a roller usually serve as counterweight for the hydraulic jack

Diameter = 300mm

Load	Load	Load	Stress	Final	Final	Final	Sett. 1	Sett. 2	Sett. 3	Avg Sett.
kg/cm ²	Bar	kN	MPa	mm	mm	mm	mm	mm	mm	mm
0.000	0.0	0.000	0.00	20.90	20.00		0.000	0.000		0.000
1.000	2.1	0.707	0.01	19.85	19.90		0.150	0.100		0.125
2.000	4.1	1.414	0.02	19.63	19.60		0.370	0.400		0.385
3.000	6.2	2.121	0.03	19.41	19.40		0.590	0.600		0.595
4.000	8.3	2.828	0.04	19.22	19.21		0.780	0.790		0.785
5.000	10.4	3.535	0.05	19.10	19.04		0.900	0.900		0.930
6.000	12.5	4.242	0.06	19.01	18.94		0.990	1.000		1.025
7.000	14.6	4.949	0.07	18.90	18.80		1.100	1.200		1.150
8.000	16.7	5.656	0.08	18.94	18.85		1.060	1.150		1.105
9.000	18.8	6.363	0.09	18.94	18.90		0.960	1.100		1.030
10.000	20.9	7.070	0.10	18.97	18.91		0.830	0.890		0.860
11.000	23.0	7.777	0.11	18.97	18.91		0.830	0.890		0.860
12.000	25.1	8.484	0.12	18.94	18.90		0.870	1.030		0.950
13.000	27.2	9.191	0.13	18.98	18.93		0.920	1.070		0.995
14.000	29.3	9.898	0.14	18.90	18.87		1.000	1.130		1.065
15.000	31.4	10.605	0.15	18.92	18.81		1.080	1.190		1.135

	σ	δS	$\delta \sigma$
0.7 σ_1	0.15	0.81582	0.34667
0.3 σ_1	0.15	0.58875	
0.7 σ_2	0.25	1.08056	0.16855
0.3 σ_2	0.15	0.82	
D (mm)	300		
E_v	129.72		
E_v	280.28		
Area (sq.m)	0.07065		

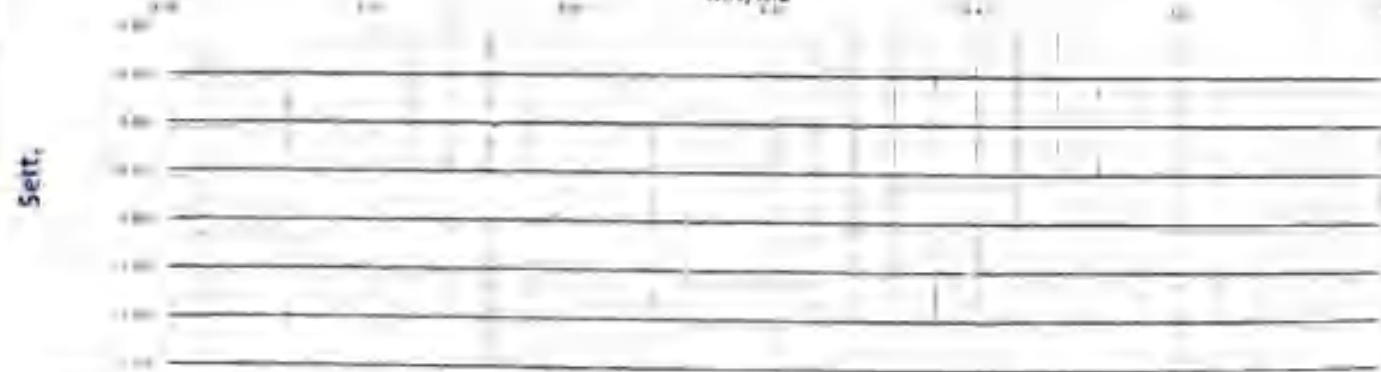
$E_v = 0.7 \sigma_1$ 2.18

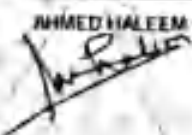
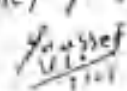
$$E_v = 0.7 \sigma_1 - D - \delta \sigma / \delta S$$

- E_v = deformation modulus
- D = load increment
- $\delta \sigma$ = settlement increment
- D = diameter of the plate, generally 0.30 m

For this calculation $\delta \sigma$ and δS are security values from the load steps between 0.3 σ_{max} and 0.7 σ_{max} .

MN/m²



Lab Specialist Name: _____ Sign: _____	Lab Engineer Name: AHMED HALEEM Sign: 	Consultant Engineer Name: Youssef Rajab Sign: 
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ENGINEERING CONSULTANCY OFFICE
Ministry of Public Works and Urban Planning
State of Palestine



CENTRAL LAB



CONSULTANT



CLIENT

Order Conditions	Customer Description	CENTRAL LAB		Contractor	Order
Plate Load Test Results					
Company Name	AL MOSTAFA				
Location	524 + 640	To	524 + 760	<div style="border: 1px solid black; padding: 2px;"> Station: 524+685 </div>	
Test Date	1-10-2023				
Layer level	SUB BALLAST +0.90				

EQUIPMENT AND TEST PROCEDURE :-

The diameter D of the plate is generally 0.30 m. For very coarse grained material also plates with diameter $D = 0.80$ m and $D = 0.753$ m are used.

The load is applied in 8 load increments of equal size. Under each load step the settlement must come to a noticeable end (< 0.02 mm/minute). After the maximum load is reached the unloading procedure can begin. After that, the plate is reloaded in 5 steps. A loaded truck, an excavator or a roller usually serve as counterweight for the hydraulic jack.

Diameter = 300mm

Load	Load	Load	Settle	Dist 1	Dist 2	Dist 3	Sett 1	Sett 2	Sett 3	Avg Sett
Stage No.	Bar	KN	mm/min	mm	mm	mm	mm	mm	mm	mm
0.000	0.0	0.000	0.00	20.00	20.00		0.000	0.000		0.000
1.000	2.1	0.707	0.01	19.85	19.95		0.050	0.050		0.050
2.000	17.1	5.652	0.08	18.70	19.86		0.300	0.140		0.220
3.000	34.2	11.304	0.16	19.40	19.52		0.400	0.480		0.440
4.000	51.3	17.863	0.25	19.57	19.30		0.430	0.700		0.565
5.000	70.5	23.315	0.33	19.15	19.70		0.650	0.800		0.725
6.000	89.8	29.873	0.42	19.25	19.07		0.750	0.970		0.840
7.000	108.8	35.325	0.50	19.15	18.93		0.850	1.070		0.950
8.000	127.8	41.883	0.58	19.16	18.90		0.920	1.020		0.920
9.000	146.7	48.431	0.67	19.21	18.84		0.790	0.960		0.875
10.000	165.7	54.979	0.75	19.33	19.15		0.670	0.850		0.760
11.000	184.7	61.527	0.83	19.35	19.15		0.670	0.850		0.760
12.000	203.7	68.075	0.91	19.32	19.14		0.680	0.860		0.770
13.000	222.7	74.623	0.99	19.26	19.07		0.740	0.930		0.835
14.000	241.7	81.171	1.07	19.21	19.01		0.790	0.890		0.800
15.000	260.7	87.719	1.15	19.18	18.95		0.820	1.050		0.935
16.000	279.7	94.267	1.23	19.15	18.91		0.850	1.090		0.970

Load	Load	Load	Settle	Dist 1	Dist 2	Dist 3	Sett 1	Sett 2	Sett 3	Avg Sett
0.7 σ_c	0.25	0.735	0.3335	0.2						
0.3 σ_c	0.15	0.4125								
0.7 σ_c	0.35	0.9675	0.16275	0.2						
0.3 σ_c	0.15	0.79								
D (mm)	300									
E_v	129.27									
E_v	276.45									
Area (sq.m)	0.0706									

σ_c / σ_{c1}	1.00		
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$$E_v = 0.75 \cdot D \cdot \sigma_c / \Delta s$$

E_v = deformation modulus

D = load increment

Δs = settlement increment

D = diameter of the plate, generally 0.30 m

For this calculation σ_{c1} and Δs_1 are usually taken from the load step between 0.2 σ_{c1} and 0.7 σ_{c1} .

MM/MPa

Sett.

Lab. Specialist

Name :

Sign :

Lab. Engineer

Name : AHMED HALEEM

Sign :

Consultant Engineer

Name : F. AL-SAYED

Sign :

مشروع: أعمال الجسر الترابي والاعمال الصناعية لمشروع القطار الكهربائي السريع
(العين السخنة - العاصمة الادارية - العلمين - مطروح) قطاع فوكة مطروح

محضر تحديد مسافة نقل (الأتربة)

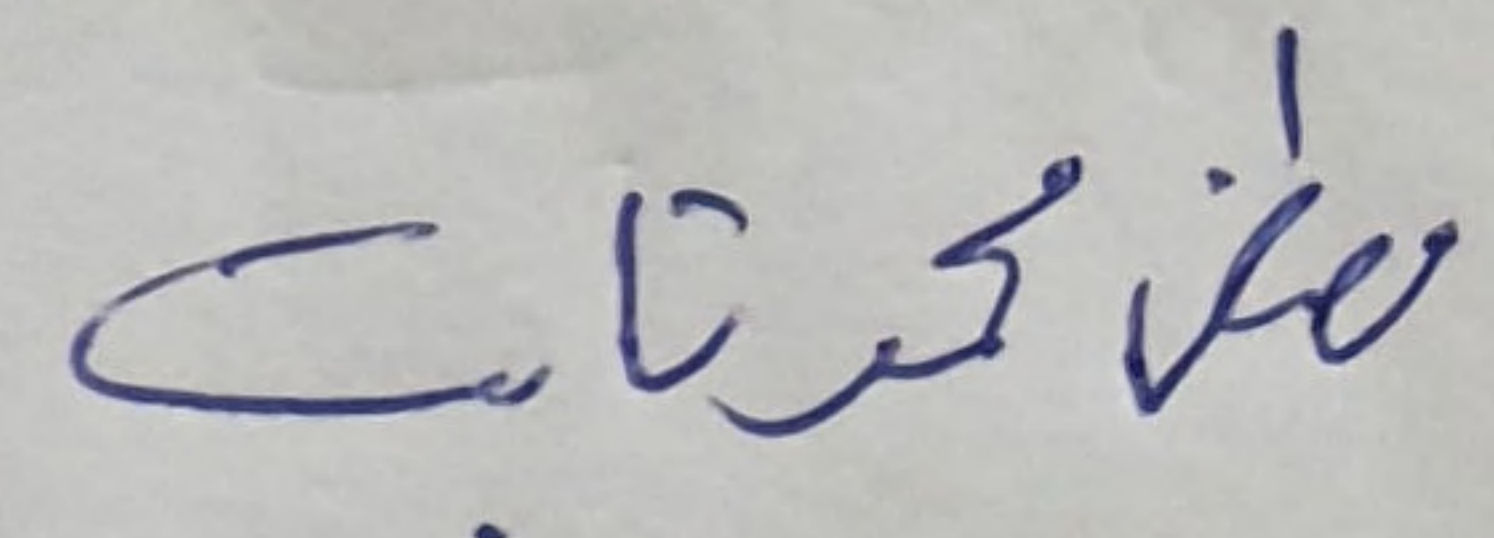
أنه في يوم الأحد الموافق 2023/9/10 وبناءً على طلب شركة المصطفى للمقاولات لتحديد مسافة نقل الأتربة من محجر (المصرية) على طريق وادي النظرون العلمين للمشروع المذكور أعلاه تم زيارة المحجر من قبل :

1. المهندس / حسن عبدالسلام سليمان مهندس جيولوجي مكتب د. خالد قنديل
2. المهندس / مصطفى محمد ثابت مدير مشروع شركة المصطفى للمقاولات

وتبين أن المحجر على مسافة 302.5 كم من منتصف قطاع شركة المصطفى للمقاولات

إحداثي المحجر : E 29° 45' 06.7" N 30° 33' 19.7"

وعلى ذلك تم التوقيع,,,

2. 

1. حسن عبد السلام حسن

مشروع: أعمال الجسر الترابي والاعمال الصناعية لمشروع القطار الكهربائي السريع
(العين السخنة - العاصمة الادارية - العلمين - مطروح) قطاع فوكة مطروح

محضر تحديد مسافة نقل (طبقة التأسيس)

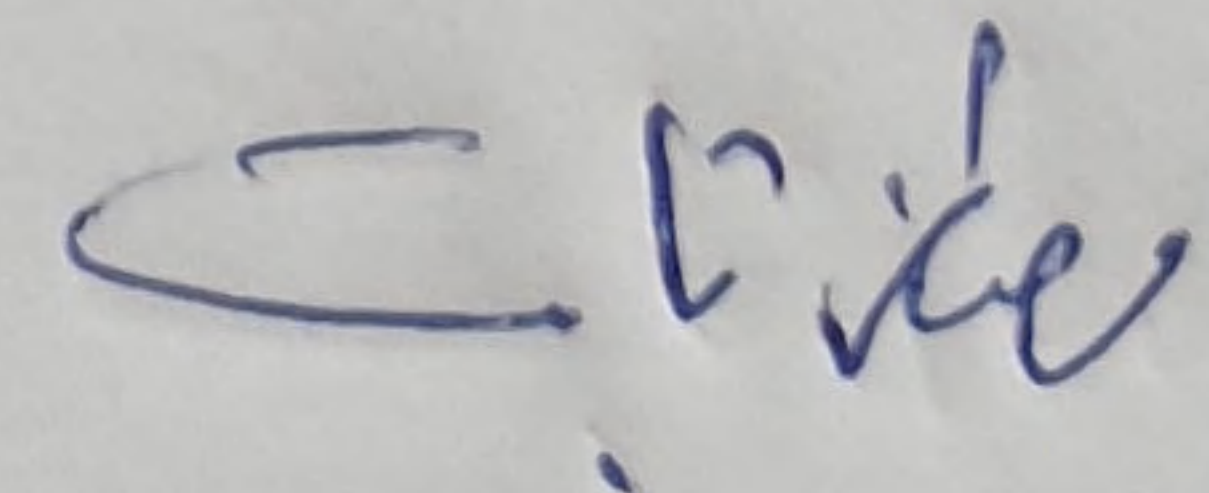
انه في يوم الأحد الموافق 2023/9/10 وبناءً على طلب شركة المصطفى للمقاولات لتحديد مسافة نقل
طبقة التأسيس للمشروع المذكور أعلاه تم زيارة الكسارة من قبل :

1. المهندس / أحمد أبوزيد مهندس جيولوجي مكتب د. خالد قنديل
2. المهندس / مصطفى محمد ثابت مدير مشروع شركة المصطفى للمقاولات

وتبين أن الكسارة على مسافة 83 كم من منتصف قطاع شركة المصطفى للمقاولات

إحداثي الكسارة : E 29° 42' 28" N 36° 38' 33"

وعلى ذلك تم التوقيع,,,

2. 

1. أحمد أبوزيد

مشروع: أعمال الجسر الترابي والاعمال الصناعية لمشروع القطار الكهربائي السريع
(العين السخنة - العاصمة الادارية - العلمين - مطروح) قطاع فوكة مطروح

محضر تحديد مسافة نقل (طبقة الأساس)

أنه في يوم الأحد الموافق 2023/9/10 وبناءً على طلب شركة المصطفى للمقاولات لتحديد مسافة نقل
طبقة الأساس للمشروع المذكور أعلاه تم زيارة الكسارة من قبل :

- | | |
|------------------------------|-----------------------------------|
| 1. المهندس / عبدالله سامي | مهندس جيولوجي مكتب د. خالد قنديل |
| 2. المهندس / مصطفى محمد ثابت | مدير مشروع شركة المصطفى للمقاولات |

وتبين أن الكسارة على مسافة 233 كم من منتصف قطاع شركة المصطفى للمقاولات

إحداثي الكسارة : E 29° 42' 28" N 36° 38' 33"

وعلى ذلك تم التوقيع,,,

2. م. محمد ثابت

1. عبدالله سامي